

MS RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	1

CREEKSIDE DRIVE (MS 4065) OVER SPRING BROOK NO. 1 BRIDGE REPLACEMENT

FOR CITY OF WHEATON DUPAGE COUNTY, IL

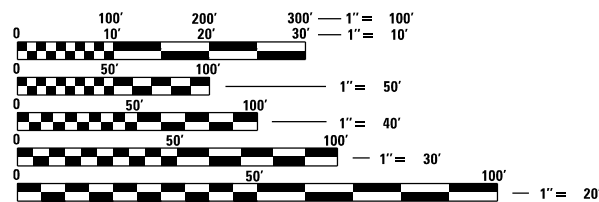
JANUARY 2023



FOR INDEX OF SHEETS, SEE SHEET NO. 2
FOR INDEX OF HIGHWAY STANDARDS, SEE SHEET NO. 2

TRAFFIC DATA

CREEKSIDE DRIVE: LOCAL ROAD
POSTED SPEED LIMIT = 25 MPH
DESIGN SPEED LIMIT = 30 MPH
2020 ADT = 2,050
2032 ADT = 2,330



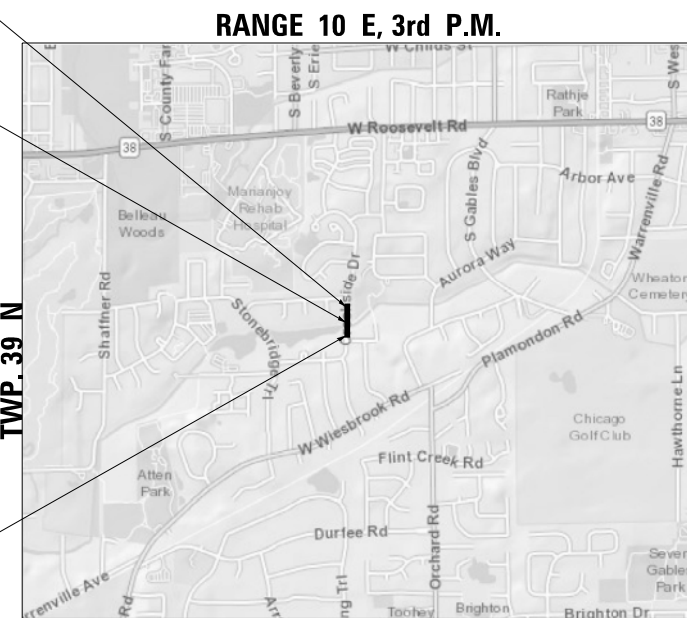
FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

J.U.L.I.E.
JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION
1-800-892-0123
OR 811

**END PROJECT
STA. 101 + 50.00**

**PR S.N. 022-7205
STA 99 + 97.00**

**BEGIN PROJECT
STA. 98 + 40.00**



LOCATION SKETCH

NOT TO SCALE

GROSS LENGTH = 310 FT. = 0.06 MILE
NET LENGTH = 310 FT. = 0.06 MILE

SA STRAND ASSOCIATES
1170 SOUTH HOUBOLT ROAD
JOLIET, ILLINOIS 60431
(815) 744-4200
IDFPR NO. 184-001273

STRAND ASSOCIATES, INC.
ALEXANDER M. SCHWARZ, P.E.
NO. 062-070948

DATE: _____ EXP: _____

STRAND ASSOCIATES, INC.
ANTHONY J. STANDISH, P.E., S.E.
NO. 081-005819

DATE: _____ EXP: _____

INDEX OF SHEETS

1	TITLE SHEET
2	INDEX OF SHEETS AND STANDARDS
3-4	SUMMARY OF QUANTITIES
5-6	TYPICAL SECTIONS
7	ALIGNMENT, TIES, AND BENCHMARKS
8	REMOVAL PLAN
9	ROADWAY PLAN AND PROFILE
10	DETOUR PLAN
11	UTILITY PLAN AND PROFILE
12-13	LANDSCAPING AND EROSION CONTROL PLAN
14	BASIN GRADING DETAILS
15	CURB RAMP DETAILS
16	BRIDGE GENERAL PLAN AND ELEVATION
17	BRIDGE GENERAL DATA
18-33	BRIDGE DETAILS
34-35	SOIL BORING LOGS
36-43	LOCAL AND DISTRICT STANDARDS
44-47	ROADWAY CROSS SECTIONS

HIGHWAY STANDARDS

000001-08	STANDARD SYMBOLS, ABBREVIATIONS AND PATTERNS
001001-02	AREAS OF REINFORCEMENT BARS
001006	DECIMAL OF AN INCH AND OF A FOOT
280001-07	TEMPORARY EROSION CONTROL SYSTEMS
420406	PAVEMENT CONNECTOR (HMA) FOR BRIDGE APPROACH SLAB
424016-05	MID-BLOCK CURB RAMPS FOR SIDEWALKS
515001-04	NAME PLATES FOR BRIDGES
542301-03	PRECAST REINFORCED CONCRETE FLARED END SECTION
606001-08	CONCRETE CURB TYPE B AND COMBINATION CONCRETE CURB AND GUTTER
701006-05	OFF-RD OPERATIONS, 2L, 2W, 15' TO 24" FROM PAVEMENT EDGE
701301-04	LANE CLOSURE, 2L, 2W, SHORT TIME OPERATIONS
701801-06	SIDEWALK, CORNER OR CROSSWALK CLOSURE
701901-08	TRAFFIC CONTROL DEVICES
720001-01	SIGN PANEL MOUNTING DETAILS
720006-04	SIGN PANEL ERECTION DETAILS
720011-01	METAL POSTS FOR SIGNS, MARKERS, & DELINEATORS

DISTRICT ONE STANDARDS

BD-02	DRIVEWAY DETAILS - DISTANCE BETWEEN ROW AND FACE OF CURB < 15' (4.5 m)
BD-32	BUTT JOINT AND HMA TAPER DETAILS
TC-13	DISTRICT ONE TYPICAL PAVEMENT MARKINGS
TC-21	DETOUR SIGNING FOR CLOSING STATE HIGHWAYS

GENERAL NOTES

- ALL CONSTRUCTION SHALL BE DONE IN ACCORDANCE WITH THE DETAILS IN THE PLANS, THE SPECIAL PROVISIONS INCLUDED IN THE CONTRACT DOCUMENTS, THE STATE OF ILLINOIS "STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION" (STANDARD SPECIFICATIONS) ADOPTED JANUARY 1, 2022, THE "SUPPLEMENTAL SPECIFICATIONS AND RECURRING SPECIAL PROVISIONS" ADOPTED JANUARY 1, 2023, AND THE LATEST EDITION OF THE FOLLOWING STATE OF ILLINOIS SPECIFICATIONS: THE "MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES FOR STREETS AND HIGHWAYS", THE "MANUAL OF TEST PROCEDURES FOR MATERIALS" AND THE "STANDARD SPECIFICATIONS FOR WATER AND SEWER MAIN CONSTRUCTION IN ILLINOIS".
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OFF ALL UNDERGROUND OR SURFACE UTILITIES, INCLUDING SPRINKLER SYSTEMS, EVEN THOUGH THEY MAY NOT BE SHOWN ON THE PLANS. THE CONTRACTOR SHALL ALSO VERIFY THE DEPTHS OF THE EXISTING UTILITIES IF NECESSARY TO VERIFY THAT GRADE CONFLICTS WILL NOT OCCUR WITH ANY PROPOSED UTILITIES PRIOR TO CONSTRUCTION AND ORDERING ANY MATERIALS. ANY RELOCATION OR LOWERING OF UTILITIES SHALL BE COORDINATED BY THE CONTRACTOR. THE COST OF THIS EXPLORATION SHALL BE INCLUDED IN THE COST OF THE PROPOSED CONSTRUCTION.
- THE CONTRACTOR SHALL NOTIFY THE CITY'S DIRECTOR OF PUBLIC WORKS AT LEAST 48 HOURS IN ADVANCE OF BEGINNING WORK TO OBTAIN CITY UTILITY LOCATIONS.
- THE CONTRACTOR MAY OBTAIN MUNICIPAL WATER IN BULK, AT NO CHARGE, AS LONG AS THERE IS NOT A "WATERING BAN" IN EFFECT. THE INDISCRIMINATE USE OF FIRE HYDRANTS IS STRICTLY PROHIBITED. WATER FOR CONSTRUCTION SHALL BE METERED OR OTHERWISE ACCOUNTED FOR AND A DAILY LOG MAINTAINED. THE CONTRACTOR SHALL PROVIDE THE WATER TRUCK AND DRIVER REQUIRED TO OBTAIN AND TRANSPORT THIS WATER. THE VILLAGE RESERVES THE RIGHT TO RESTRICT OR REFUSE THE USE OF VILLAGE WATER IF DEEMED NECESSARY.
- ACCESS TO PRIVATE DRIVEWAYS SHALL BE PROVIDED AT ALL TIMES EXCEPT DURING ACTUAL CONSTRUCTION ADJACENT THERE TO. TEMPORARY RAMPS SHALL BE CONSTRUCTED AS NEEDED TO PROVIDE SUCH ACCESS UTILIZING CRUSHED STONE OR CRUSHED GRAVEL AS TEMPORARY ACCESS.
- IT WILL BE THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE ENGINEER, RESIDENTS AND THE VILLAGE WHEN ACCESS TO DRIVEWAYS WILL BE TEMPORARILY CLOSED DUE TO CURB AND GUTTER AND/OR DRIVEWAY REPLACEMENT. THE CONTRACTOR SHALL DISTRIBUTE NOTICES PROVIDED BY THE VILLAGE TO RESIDENTS AT LEAST 48 HOURS PRIOR TO PLANNED CLOSURE. EVERY EFFORT SHALL BE MADE TO ACCOMMODATE ACCESS TO THESE PROPERTIES INCLUDING KNOCKING ON DOORS WHEN DRIVEWAYS ARE ABOUT TO BE CLOSED.
- PORTLAND CEMENT CONCRETE SIDEWALK SHALL BE THICKENED TO 6 INCHES AT LOCATIONS WHERE THE SIDEWALK CROSSES DRIVEWAYS. TRANSVERSE EXPANSION JOINTS 3/4" SHALL BE PLACED EVERY 50 FEET OR AS DETERMINED BY THE ENGINEER. TRANSVERSE CONTRACTION JOINTS SHALL BE PLACED EVERY 5 FEET.
- A 1/2-INCH-THICK EXPANSION JOINT SHALL BE PROVIDED AT THE JUNCTION OF THE DRIVEWAY APRON AND CURB, AND AT THE JUNCTION OF THE DRIVEWAY APRON AND SIDEWALK.
- THE CONTRACTOR SHALL CONTACT THE LOCAL AGENCY MATERIAL INSPECTOR AT LEAST 48 HOURS PRIOR TO ANY CONCRETE OR HOT-MIX ASPHALT MATERIAL DELIVERIES.
- IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND CONDITIONS EXISTING IN THE FIELD PRIOR TO CONSTRUCTION AND ORDERING OF MATERIALS.
- THE LOCATION AND ELEVATION OF THE VARIOUS UNDERGROUND UTILITIES AS SHOWN ON THE DRAWINGS ARE APPROXIMATE. THE CONTRACTOR SHALL USE SPECIAL CARE WHEN CONDUCTING CONSTRUCTION OPERATIONS NEAR THEM AND IS RESPONSIBLE FOR ANY DAMAGE CAUSED.
- THE CONTRACTOR SHALL NOTIFY ALL AFFECTED AGENCIES AND UTILITIES AT LEAST 10 DAYS PRIOR TO ANY CONSTRUCTION IN THE AREA AND SHALL COMPLY WITH ALL RESTRICTIONS FOR EQUIPMENT MOVEMENTS AND CLEARANCES IN REGARDS TO THEIR FACILITIES.
- THE CONTRACTOR SHALL OBTAIN ALL PERMITS AS LISTED IN THE SPECIAL PROVISIONS PRIOR TO COMMENCEMENT OF WORK.
- WHERE SECTION OR SUBSECTION MONUMENTS ARE ENCOUNTERED, THE ENGINEER SHALL BE NOTIFIED BEFORE SUCH MONUMENTS ARE REMOVED. THE CONTRACTOR SHALL PROTECT AND CAREFULLY PRESERVE ALL PROPERTY MARKER MONUMENTS UNTIL THE OWNER, AN AUTHORIZED AGENT, OR LAND SURVEYOR HAS WITNESSED OR OTHERWISE REFERENCED THEIR LOCATION. THE CONTRACTOR WILL BE RESPONSIBLE FOR HAVING AN AUTHORIZED SURVEYOR REESTABLISH ANY SECTION OR SUB-SECTION MONUMENTS DISRUPTED BY THEIR OPERATIONS.
- ALL TRAFFIC SIGNS, STREET SIGNS, ETC., THAT INTERFERE WITH THE CONSTRUCTION OPERATIONS SHALL BE REMOVED AND PLACED AT NEW LOCATIONS AS DESIGNATED BY THE ENGINEER. IN ADDITION, ALL MAIL BOXES THAT INTERFERE WITH CONSTRUCTION SHALL BE SIMILARLY RELOCATED.
- THE DAYS PAVING OPERATION SHOULD RESULT IN A SINGLE TRANSVERSE JOINT. ANY COLD LONGITUDINAL JOINTS WILL NOT BE ACCEPTED. PROVIDING A SINGLE TRANSVERSE JOINT SHALL BE ACCOMPLISHED BY PAVING ONE LANE OF SUFFICIENT LENGTH THAT WILL ALLOW FOR THE PAVING OF THE ADJACENT LANE IN THE SAME DAY.

- DETECTABLE WARNINGS SHALL BE CONSTRUCTED WITH THE INSTALLATION OF A CAST-IN-PLACE 'ARMOR-TILE' 24"x60" NOMINAL PANEL WIDTH AS MANUFACTURED BY 'ENGINEERED PLASTICS, INC.' (800) 682 2525 OF WILLIAMSVILLE, NY. THE PANEL SHALL BE A POLYMER COMPOSITE AND COMPLY WITH ADA REQUIREMENTS. THE DOMES LOCATED ON THE PANEL SHALL PARALLEL THE PAVEMENT CROSS WALK WITH THE CLOSEST EDGE LOCATED AT THE BACK OF CURB. THE CONTRASTING PANEL COLOR SHALL BE SELECTED BY THE CITY. INSTALLATION SHALL OCCUR IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
- IN AREAS WHERE THE EXISTING DRIVEWAY, SIDEWALK, OR CURB AND GUTTER IS TO BE REMOVED AND REPLACED, THE REMOVAL AND DISPOSAL OF ANY ADDITIONAL MATERIAL REQUIRED TO ESTABLISH THE PROPOSED DIRVEWAY, SIDEWALK, OR CURB AND GUTTER SUBGRADE ELEVATION SHALL BE INCLUDED IN THE APPROPRIATE REMOVAL PAY ITEMS.
- THE CONTRACTOR WILL BE REQUIRED TO USE A STEEL PLATE OR PLATES TO CLOSE ANY GAPS OCCURRING WHEN A FRAME IS OFFSET FROM THE STRUCTURE. THE STEEL PLATE SHALL BE 1/2" THICK AND APPROXIMATELY 6-INCH WIDE BY 24-INCH LONG. SOME ADJUSTMENT IN SIZE MAY BE NECESSARY TO PREVENT THE STEEL PLATE FROM OVERHANGING THE OUTSIDE OF THE STRUCTURE WALL. THE STEEL PLATE SHALL BE BEDDED IN AND COVERED WITH MORTAR.
- A PORTABLE BATHROOM(S) SHALL BE PLACED ON THE JOB SITE(S) AND RELOCATED WHEN NECESSARY SO IT IS ACCESSIBLE TO WORKERS. IF WORK IS OCCURRING AT SEVERAL LOCATIONS, ONE PORTABLE BATHROOM SHALL BE PLACED AT EACH LOCATION WITHIN A REASONABLE DISTANCE FROM THE WORK AS DETERMINED BY THE ENGINEER.
- THOSE SEEKING THE FULL HYDRAULIC REPORT SHOULD CONTACT THE OWNER OF RECORD. TO MAKE ARRANGEMENTS FOR ACCESS TO THIS INFORMATION PLEASE CONTACT THE WHEATON DEPARTMENT OF ENGINEERING AT 630-260-2065.
- IN ALL INSTANCES WHERE PAVEMENT SAW CUTS ARE REQUIRED, THEY SHALL BE CUT AT LANE LINES AND PERPENDICULAR TO THE DIRECTION OF TRAFFIC.
- THE CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS FOR THE PROTECTION OF EXISTING PLANT MATERIAL FOR WHICH THE CONTRACT DOES NOT PROVIDE REMOVAL. THE PROTECTION OF EXISTING PLANT MATERIAL AND THE REPAIR OR REPLACEMENT OF EXISTING PLANT MATERIAL DAMAGED BY THE CONTRACTOR SHALL BE DONE IN ACCORDANCE WITH THE REQUIREMENTS OF SECTION 201 OF THE STANDARD SPECIFICATIONS.
- THE CONTRACTOR SHALL MAINTAIN POSITIVE DRAINAGE AT ALL TIMES DURING AND AFTER CONSTRUCTION.
- WHEN EXISTING DRAINAGE FACILITIES ARE DISTURBED, THE CONTRACTOR SHALL PROVIDE AND MAINTAIN TEMPORARY OUTLETS AND CONNECTIONS FOR ALL PRIVATE OR PUBLIC DRAINS, SEWERS, OR CATCH BASINS. CONTRACTOR SHALL PROVIDE FACILITIES TO TAKE IN ALL STORM WATER WHICH WILL BE RECEIVED BY THESE DRAINS AND SEWERS AND DISCHARGE THEM. CONTRACTOR SHALL PROVIDE AND MAINTAIN AN EFFICIENT PUMPING PLANT, IF NECESSARY, AND A TEMPORARY OUTLET SHALL BE PREPARED AT ALL TIMES TO DISPOSE OF THE WATER RECEIVED FROM THESE TEMPORARY CONNECTIONS UNTIL SUCH TIME AS THE PERMANENT ROADSIDE DRAINAGE SYSTEM IS BUILT AND IN SERVICE.
- DRAINAGE STRUCTURE GRADES SHALL BE VERIFIED IN THE FIELD PRIOR TO INSTALLATION OF DRAINAGE ITEMS. DRAINAGE STRUCTURES MAY REQUIRE REVISIONS TO MEET EXISTING FIELD CONDITIONS. ANY ADJUSTMENT SHALL BE AS DIRECTED BY THE ENGINEER.
- UNLESS OTHERWISE NOTED, LOCATIONS SHOWN ON THE DRAWINGS ARE TO THE CENTER OF THE GRATE/LID FOR ALL MANHOLE STRUCTURES. FLAT TOPS AND CONES ARE TO BE TURNED SO THAT THE FRAME IS CLOSEST TO THE CENTER LINE OF THE ROAD, UNLESS OTHERWISE NOTED ON THE STRUCTURE IN THE PLANS. ALL FLAT TOPS AND CONES ARE ASSUMED TO BE ECCENTRIC.
- THE CITY OF WHEATON IS SOLELY RESPONSIBLE FOR OPENING ALL WATER VALVES AND FIRE HYDRANTS ON THE PROJECT.
- A 48 HOUR NOTICE IS REQUIRED PRIOR TO ANY WATER SHUT OFF TO NOTIFY AFFECTED AREAS. CONTRACTOR TO NOTIFY PUBLIC WORKS AT 630-260-2110, 48 HOURS IN ADVANCE OF ANY REQUIRED OPERATIONS.

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**CITY OF WHEATON
DUPAGE COUNTY, ILLINOIS**

**CREEKSIDE DRIVE OVER SPRING BROOK NO. 1
INDEX OF SHEETS, STANDARDS, AND GENERAL NOTES**

SCALE: NTS SHEET 1 OF 1 SHEETS STA. N/A TO STA. N/A

MS RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	2

CODE NO.	ITEM	UNIT	TOTAL QUANTITY
20100110	TREE REMOVAL (6 TO 15 UNITS DIAMETER)	UNIT	12
20101100	TREE TRUNK PROTECTION	EACH	5
20200100	EARTH EXCAVATION	CU YD	200.00
20300100	CHANNEL EXCAVATION	CU YD	315
21101615	TOPSOIL FURNISH AND PLACE, 4"	SQ YD	1,800
25000210	SEEDING, CLASS 2A	ACRE	0
25100630	EROSION CONTROL BLANKET	SQ YD	1,800
28000250	TEMPORARY EROSION CONTROL SEEDING	POUND	160
28000400	PERIMETER EROSION BARRIER	FOOT	885
28000510	INLET FILTERS	EACH	4
28001100	TEMPORARY EROSION CONTROL BLANKET	SQ YD	1,800
28100107	STONE RIPRAP, CLASS A4	SQ YD	460
28200200	FILTER FABRIC	SQ YD	460
31101200	SUBBASE GRANULAR MATERIAL, TYPE B 4"	SQ YD	319
31101600	SUBBASE GRANULAR MATERIAL, TYPE B 8"	SQ YD	630.00
35101800	AGGREGATE BASE COURSE, TYPE B 6"	SQ YD	28.00
35102000	AGGREGATE BASE COURSE, TYPE B 8"	SQ YD	57.00
35501309	HOT-MIX ASPHALT BASE COURSE, 6 1/4"	SQ YD	515.00
40600275	BITUMINOUS MATERIALS (PRIME COAT)	POUND	1,155.00
40600290	BITUMINOUS MATERIALS (TACK COAT)	POUND	360
40600982	HOT-MIX ASPHALT SURFACE REMOVAL - BUTT JOINT	SQ YD	27
40603080	HOT-MIX ASPHALT BINDER COURSE, IL-19.0, N50	TON	65
40604060	HOT-MIX ASPHALT SURFACE COURSE, IL-9.5, MIX "D", N50	TON	55

CODE NO.	ITEM	UNIT	TOTAL QUANTITY
42000070	PAVEMENT CONNECTOR (HMA) FOR BRIDGE APPROACH SLAB	SQ YD	62
42400200	PORTLAND CEMENT CONCRETE SIDEWALK 5 INCH	SQ FT	2,870
42400800	DETECTABLE WARNINGS	SQ FT	20
44000100	PAVEMENT REMOVAL	SQ YD	696
44000200	DRIVEWAY PAVEMENT REMOVAL	SQ YD	49
44000500	COMBINATION CURB AND GUTTER REMOVAL	FOOT	475
44000600	SIDEWALK REMOVAL	SQ FT	2,836
50100100	REMOVAL OF EXISTING STRUCTURES	EACH	1
50200100	STRUCTURE EXCAVATION	CU YD	135
50300225	CONCRETE STRUCTURES	CU YD	62
50300255	CONCRETE SUPERSTRUCTURE	CU YD	70
50300260	BRIDGE DECK GROOVING	SQ YD	327
50300280	CONCRETE ENCASEMENT	CU YD	5
50300300	PROTECTIVE COAT	SQ YD	469
50301350	CONCRETE SUPERSTRUCTURE (APPROACH SLAB)	CU YD	78
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	57,525
50900105	ALUMINUM RAILING, TYPE L	FOOT	100
51201400	FURNISHING STEEL PILES HP10X42	FOOT	520
51202305	DRIVING PILES	FOOT	520
51203400	TEST PILE STEEL HP10X42	EACH	2
51204650	PILE SHOES	EACH	12
51500100	NAME PLATES	EACH	1
52100520	ANCHOR BOLTS, 1"	EACH	28

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**CITY OF WHEATON
DUPAGE COUNTY, ILLINOIS**

**CREEKSIDE DRIVE OVER SPRING BROOK NO. 1
SUMMARY OF QUANTITIES**

SCALE: NTS SHEET 1 OF 2 SHEETS STA. N/A TO STA. N/A

MS RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	3

CODE NO.	ITEM	UNIT	TOTAL QUANTITY
56103100	DUCTILE IRON WATER MAIN 8"	FOOT	140
58600101	GRANULAR BACKFILL FOR STRUCTURES	CU YD	28
59100100	GEOCOMPOSITE WALL DRAIN	SQ YD	54
60255500	MANHOLES TO BE ADJUSTED	EACH	4
60265700	VALVE VAULTS TO BE ADJUSTED	EACH	2
67100100	MOBILIZATION	L SUM	1
72400100	REMOVE SIGN PANEL ASSEMBLY - TYPE A	EACH	2
72400500	RELOCATE SIGN PANEL ASSEMBLY - TYPE A	EACH	2
72800100	TELESCOPING STEEL SIGN SUPPORT	FOOT	30
78000600	THERMOPLASTIC PAVEMENT MARKING - LINE 12"	FOOT	48
A2002524	TREE, CARPINUS CAROLINIANA (AMERICAN HORNBEAM), 3" CALIPER, BALLED AND BURLAPPED	EACH	3
A2005030	TREE, GYMNOCLADUS DIOICUS (KENTUCKY COFFEETREE), 4" CALIPER, BALLED AND BURLAPPED	EACH	1
A2008224	TREE, TILIA X EUCHLOREA (CRIMEAN LINDEN), 3" CALIPER, BALLED AND BURLAPPED	EACH	2
B2001124	TREE, CERCIS CANADENSIS (EASTERN REDBUD), 3" CALIPER, TREE FORM, BALLED AND BURLAPPED	EACH	2
X0325950	GATE VALVE 8" WITH VAULT, 5' DIAMETER	EACH	2
X4021000	TEMPORARY ACCESS (PRIVATE ENTRANCE)	EACH	2
X4023000	TEMPORARY ACCESS (ROAD)	EACH	1
X5630008	CUT AND CAP EXISTING 8" WATER MAIN	EACH	2
X5630708	CONNECTION TO EXISTING WATER MAIN 8"	EACH	2
X6061815	COMBINATION CONCRETE CURB AND GUTTER, TYPE M (SPECIAL)	FOOT	385
X7010216	TRAFFIC CONTROL AND PROTECTION, (SPECIAL)	L SUM	1
X8360110	LIGHT POLE FOUNDATION, SPECIAL	FOOT	8
X8440110	RELOCATE EXISTING LIGHT POLE WITH LUMINAIRE	EACH	2

CODE NO.	ITEM	UNIT	TOTAL QUANTITY
Z0013798	CONSTRUCTION LAYOUT	L SUM	1
Z0046304	PIPE UNDERDRAINS FOR STRUCTURES 4"	FOOT	103
	PRESS-BRAKE-FORMED STEEL TUB GIRDER (PBFSTG)	POUND	41,619
	TEMPORARY LINE STOP 8"	EACH	2

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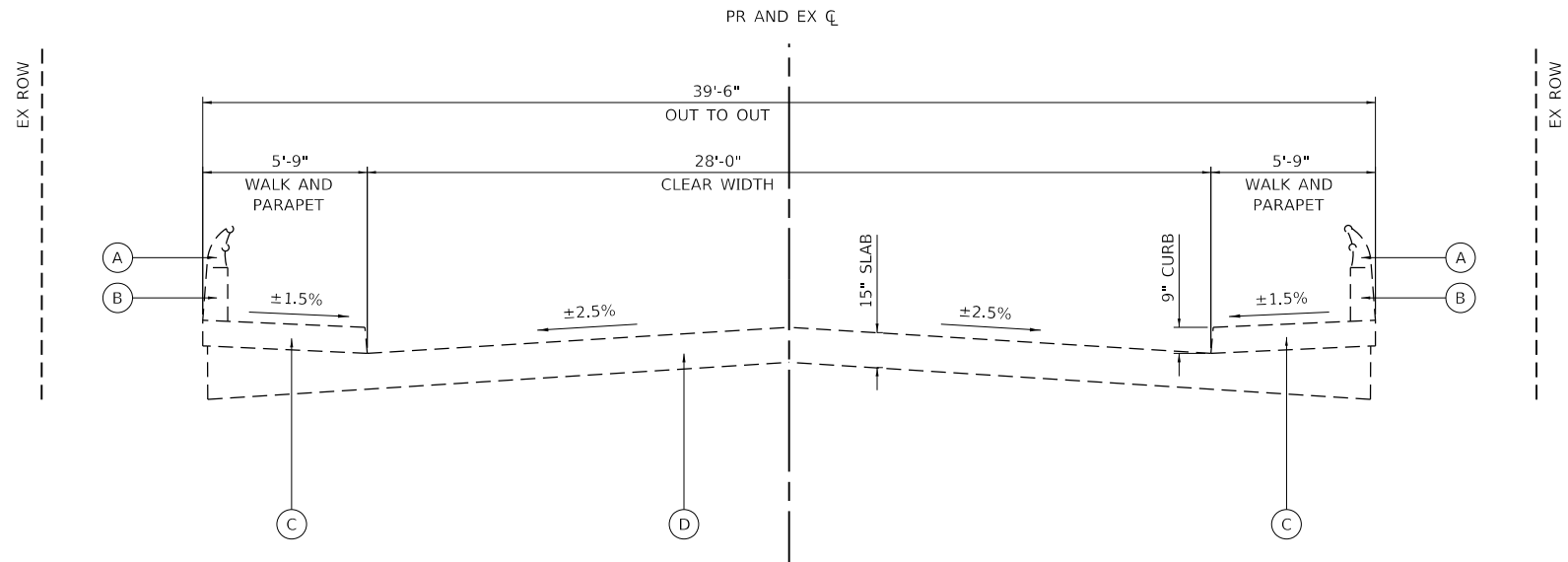
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**CITY OF WHEATON
DUPAGE COUNTY, ILLINOIS**

**CREEKSIDE DRIVE OVER SPRING BROOK NO. 1
SUMMARY OF QUANTITIES**

SCALE: NTS SHEET 2 OF 2 SHEETS STA. N/A TO STA. N/A

MS RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	4

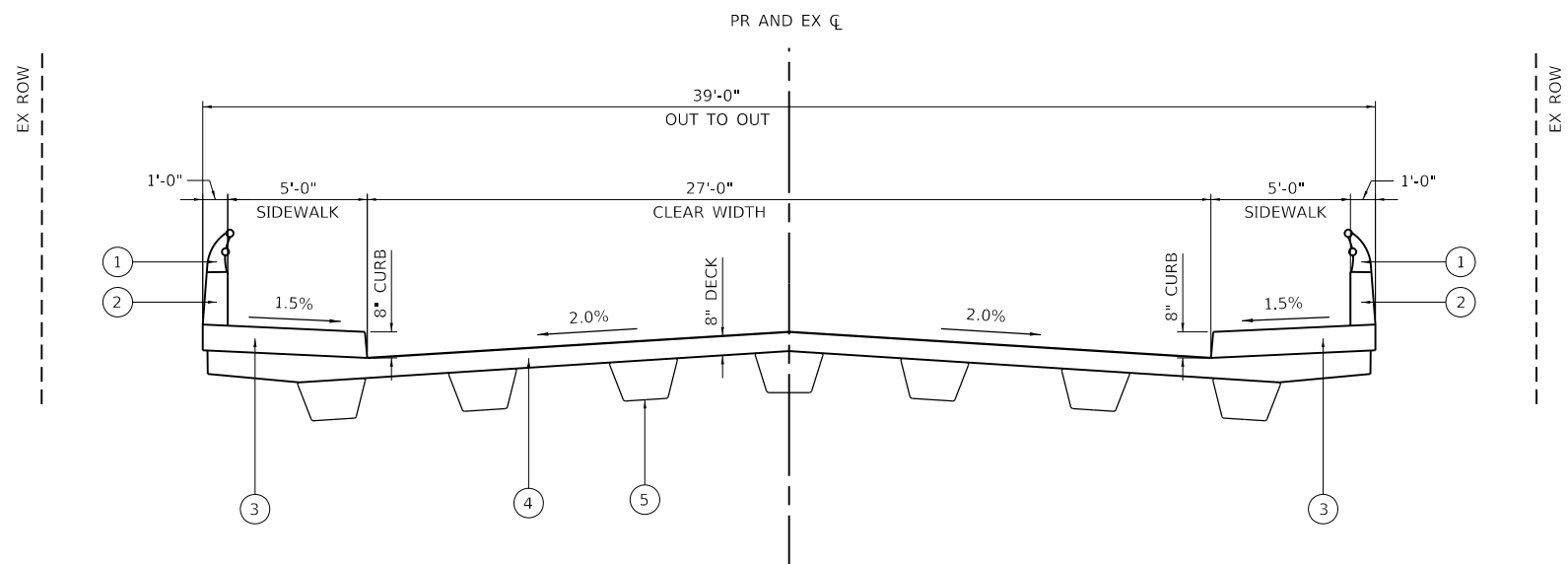


EXISTING TYPICAL SECTION

EXISTING STRUCTURE NO. 022-7202

EXISTING LEGEND

- (A) EXISTING RAILING
- (B) CONCRETE PARAPET
- (C) RAISED SIDEWALK
- (D) REINFORCED CONCRETE BRIDGE SLAB



PROPOSED TYPICAL SECTION

PROPOSED STRUCTURE NO. 022-7205
SEE STRUCTURE SHEETS FOR DETAILS

PROPOSED LEGEND

- (1) ALUMINUM RAILING, TYPE L
- (2) CONCRETE PARAPET
- (3) CONCRETE SUPERSTRUCTURE
- (4) REINFORCED CONCRETE DECK, 8"
- (5) PRESS-BRAKE-FORMED STEEL TUB GIRDER (PBFSTG)

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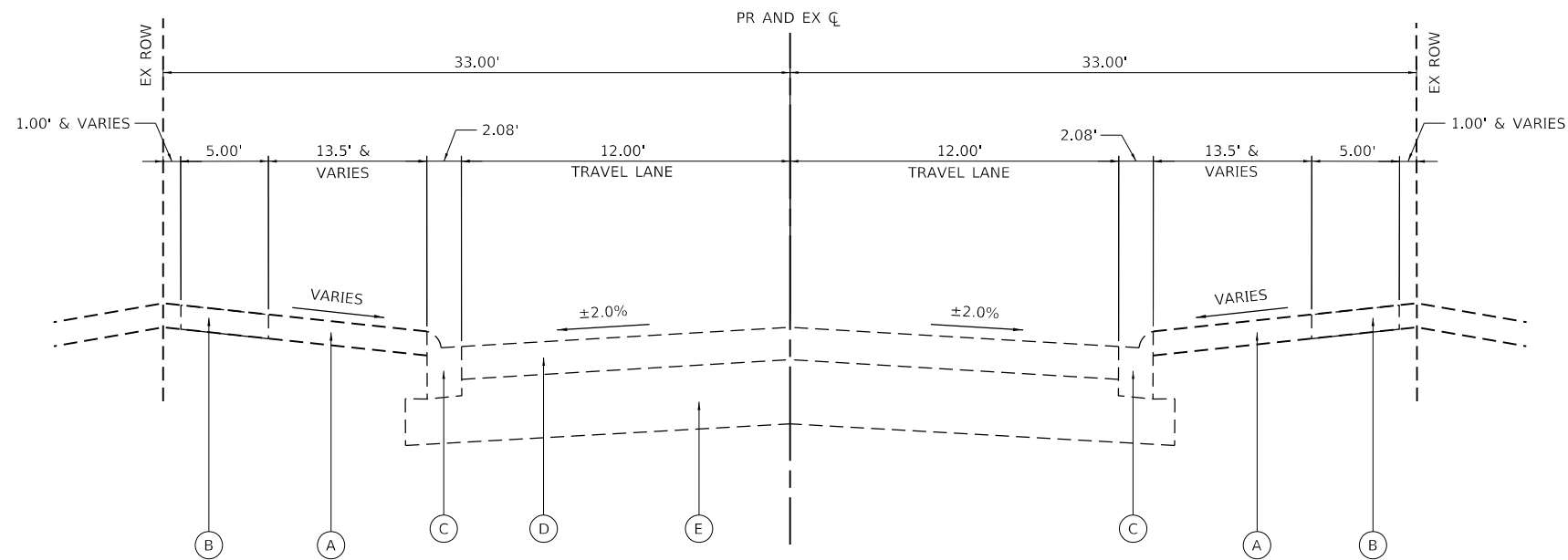
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**CITY OF WHEATON
DUPAGE COUNTY, ILLINOIS**

**CREEKSIDE DRIVE OVER SPRING BROOK NO. 1
TYPICAL SECTIONS**

SCALE: NTS SHEET 1 OF 2 SHEETS STA. N/A TO STA. N/A

MS RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	5

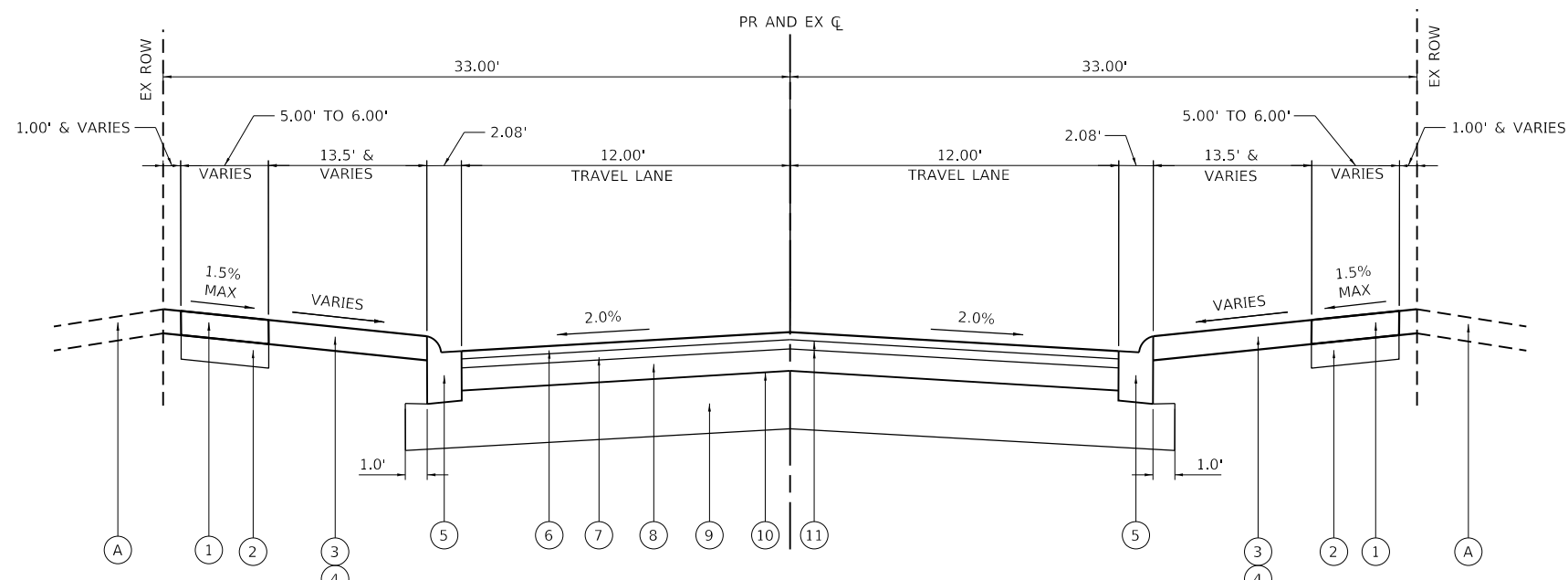


EXISTING TYPICAL SECTION

ROADWAY APPROACHES - CREEKSIDE DRIVE

EXISTING LEGEND

- (A) GROUND SURFACE
- (B) CONCRETE SIDEWALK, 5"
- (C) COMBINATION CONCRETE CURB AND GUTTER, TYPE M (SPECIAL)
- (D) EXISTING HOT-MIX ASPHALT PAVEMENT, VARIES
- (E) EXISTING AGGREGATE SUBBASE, VARIES



PROPOSED TYPICAL SECTION

ROADWAY APPROACHES - CREEKSIDE DRIVE

PROPOSED LEGEND

- (1) PORTLAND CEMENT CONCRETE SIDEWALK, 5 INCH
- (2) SUBBASE GRANULAR MATERIAL, TYPE B 4"
- (3) SEEDING, CLASS 2A
- (4) TOPSOIL FURNISH AND PLACE, 4"
- (5) COMBINATION CONCRETE CURB AND GUTTER, TYPE M (SPECIAL)
- (6) HOT-MIX ASPHALT SURFACE COURSE, IL-9.5, MIX "D", N50, 1.5"
- (7) HOT-MIX ASPHALT BINDER COURSE, IL-19.0, N50, 2.25"
- (8) HOT-MIX ASPHALT BASE COURSE, 6 1/4"
- (9) SUBBASE GRANULAR MATERIAL, TYPE B 8"
- (10) BITUMINOUS MATERIALS (PRIME COAT)
- (11) BITUMINOUS MATERIALS (TACK COAT) BETWEEN LIFTS AS NEEDED

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**CITY OF WHEATON
DUPAGE COUNTY, ILLINOIS**

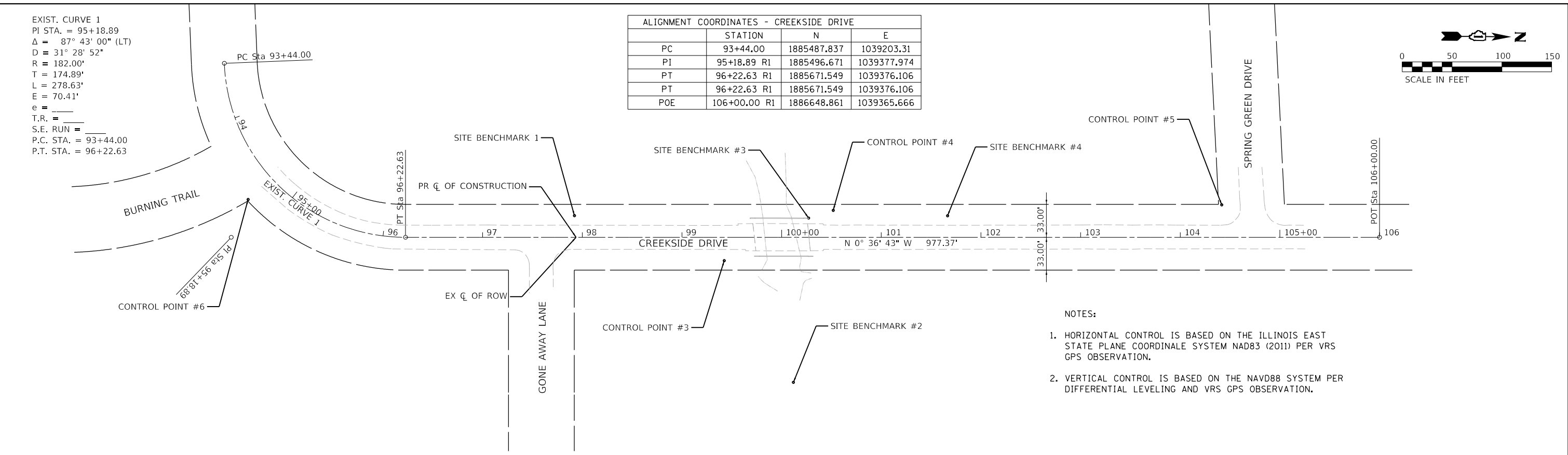
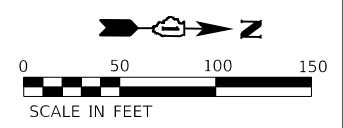
**CREEKSIDE DRIVE OVER SPRING BROOK NO. 1
TYPICAL SECTIONS**

SCALE: NTS SHEET 2 OF 2 SHEETS STA. N/A TO STA. N/A

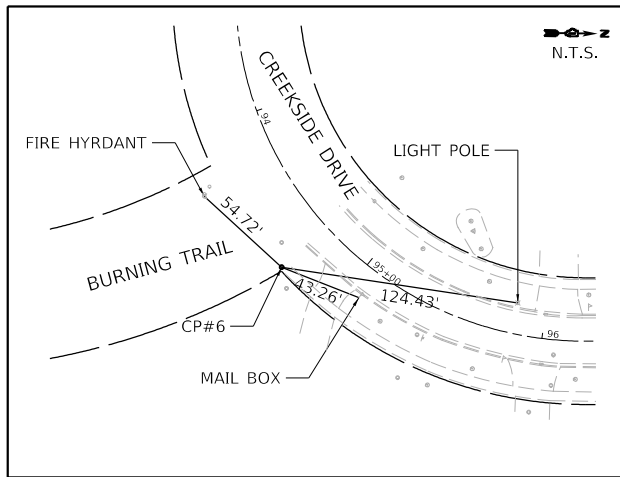
MS RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	6

EXIST. CURVE 1
 PI STA. = 95+18.89
 $\Delta = 87^\circ 43' 00''$ (LT)
 $D = 31^\circ 28' 52''$
 $R = 182.00'$
 $T = 174.89'$
 $L = 278.63'$
 $E = 70.41'$
 $e =$
 $T.R. =$
 $S.E. RUN =$
 $P.C. STA. = 93+44.00$
 $P.T. STA. = 96+22.63$

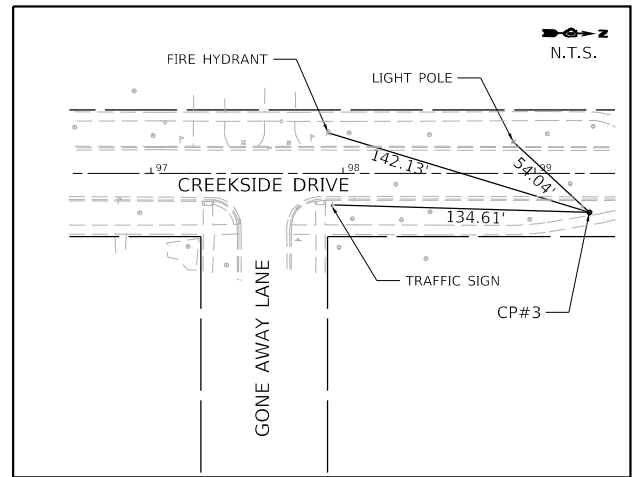
ALIGNMENT COORDINATES - CREEKSIDE DRIVE			
	STATION	N	E
PC	93+44.00	1885487.837	1039203.31
PI	95+18.89 R1	1885496.671	1039377.974
PT	96+22.63 R1	1885671.549	1039376.106
PT	96+22.63 R1	1885671.549	1039376.106
POE	106+00.00 R1	1886648.861	1039365.666



- NOTES:
- HORIZONTAL CONTROL IS BASED ON THE ILLINOIS EAST STATE PLANE COORDINATE SYSTEM NAD83 (2011) PER VRS GPS OBSERVATION.
 - VERTICAL CONTROL IS BASED ON THE NAVD88 SYSTEM PER DIFFERENTIAL LEVELING AND VRS GPS OBSERVATION.

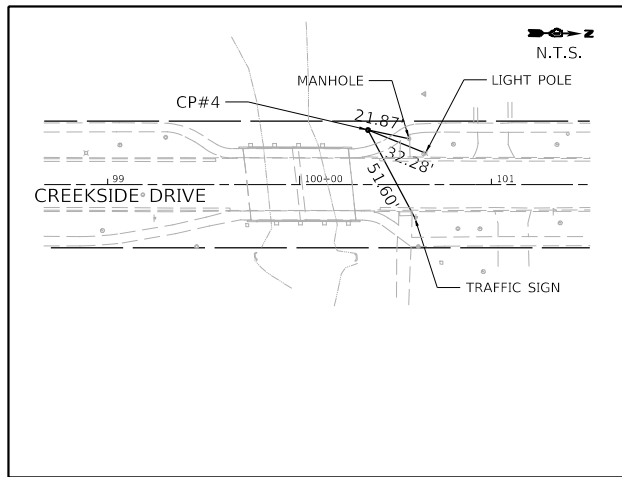


CONTROL POINT #6
 SET CUT CROSS
 IN CONC. WALK
 STA. 94+71.32, 31.79' RT
 N: 1,887,190.876
 E: 1,039,192.243
 ELEV.=728.658



CONTROL POINT #3
 SET CUT CROSS
 IN CONC. WALK
 STA. 99+42.44, 23.46' RT
 N: 1,887,669.269
 E: 1,039,248.535
 ELEV.=725.513

SITE BENCHMARK #1
 ELEVATION=725.308
 SET CUT CROSS ON WSW BOLT ON FIRE HYDRANT FLANGE
 @ THE NW COR. OF CREEKSIDE DRIVE & GONE AWAY LANE.

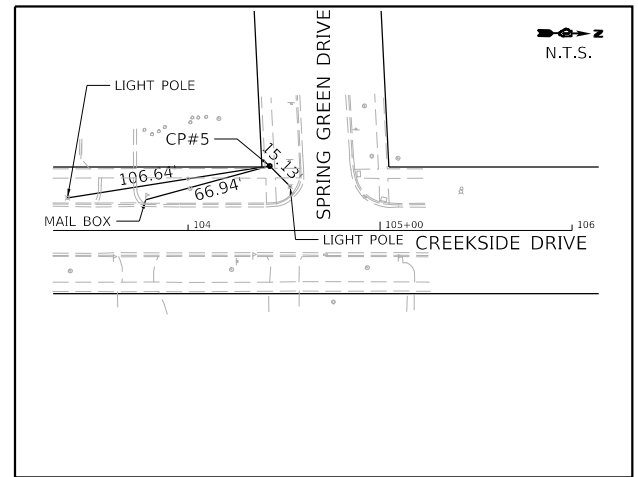


CONTROL POINT #4
 SET CUT CROSS
 IN CONC. WALK
 100+51.81, 27.06' LT
 N: 1,887,778.092
 E: 1,039,196.847
 ELEV.=725.392

SITE BENCHMARK #2
 ELEVATION=725.308
 SET CUT SQUARE @ CL OF CONC. HEADWALL @ 40" RCP
 ON S. SIDE OF SPRING BROOK CREEK E. OF CREEKSIDE DR

SITE BENCHMARK #3
 ELEVATION=729.589
 SET CUT SQUARE ON TOP OF CONC. HEADWALL
 @ THE NW COR. OF THE CREEKSIDE DRIVE BRIDGE
 OVER SPRING BROOK CREEK.

SITE BENCHMARK #4
 ELEVATION=725.370
 SET CUT CROSS ON WNW BOLT ON FIRE HYDRANT FLANGE
 ON THE W. SIDE OF CREEKSIDE DRIVE @ ADDRESS #1482/1488



CONTROL POINT #5
 SET CUT CROSS
 IN CONC. WALK
 104+41.65, 32.63' LT
 N: 1,888,167.853
 E: 1,039,187.115
 ELEV.=729.356

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





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	DATE -	REVISED -

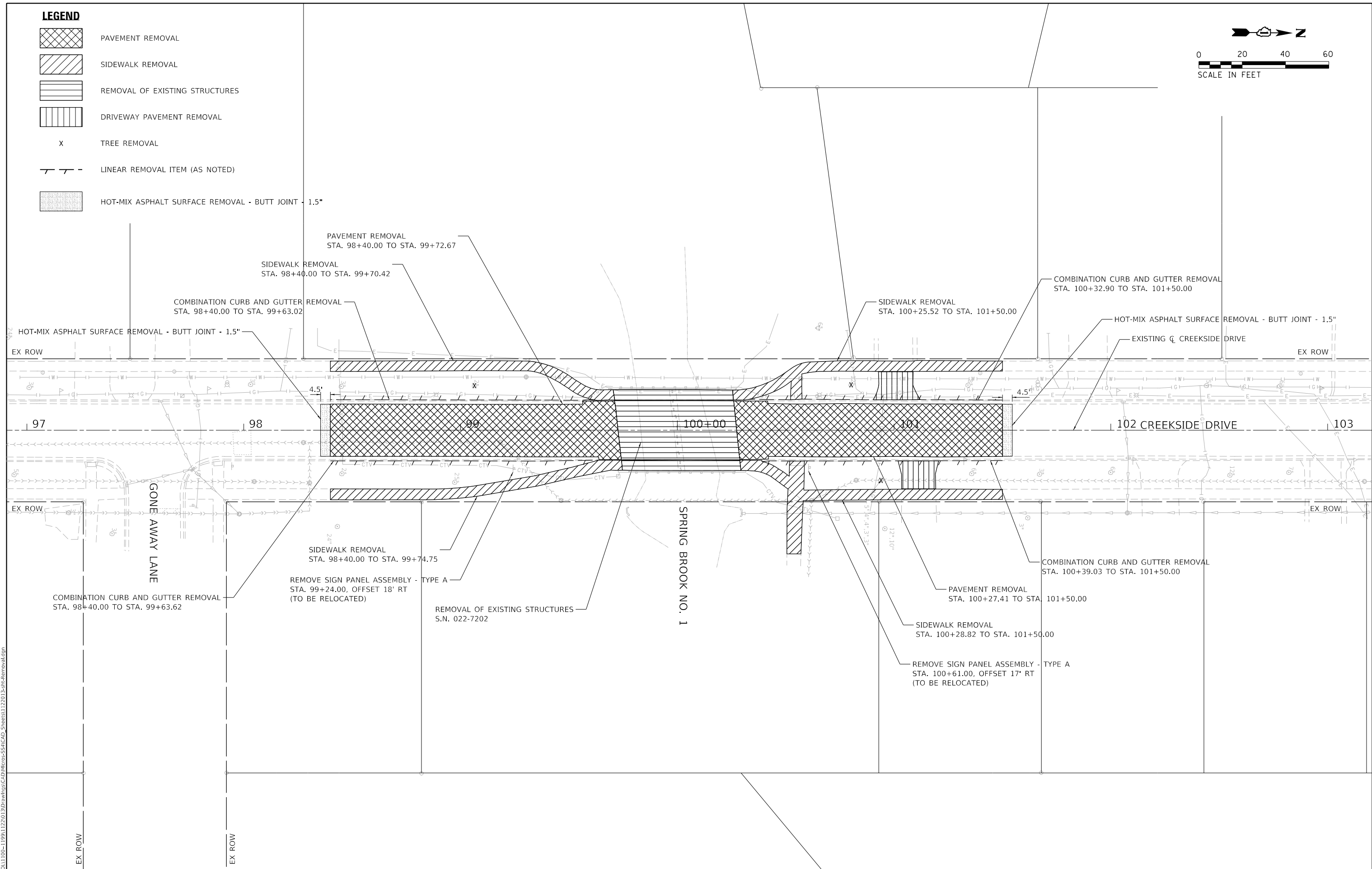
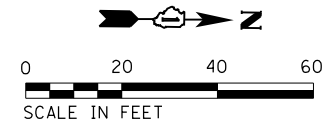
**CITY OF WHEATON
 DUPAGE COUNTY, ILLINOIS**

CREEKSIDE DRIVE OVER SPRING BROOK NO. 1 ALIGNMENT, TIES, AND BENCHMARKS	
SCALE: NTS	SHEET 1 OF 1 SHEETS
STA. N/A	TO STA. N/A

MS RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	7

LEGEND

-  PAVEMENT REMOVAL
-  SIDEWALK REMOVAL
-  REMOVAL OF EXISTING STRUCTURES
-  DRIVEWAY PAVEMENT REMOVAL
-  TREE REMOVAL
-  LINEAR REMOVAL ITEM (AS NOTED)
-  HOT-MIX ASPHALT SURFACE REMOVAL - BUTT JOINT - 1.5"



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PLOT DATE = 1/31/2023	DATE -	REVISED -

**CITY OF WHEATON
DUPAGE COUNTY, ILLINOIS**

**CREEKSIDE DRIVE OVER SPRING BROOK NO. 1
REMOVAL PLAN**

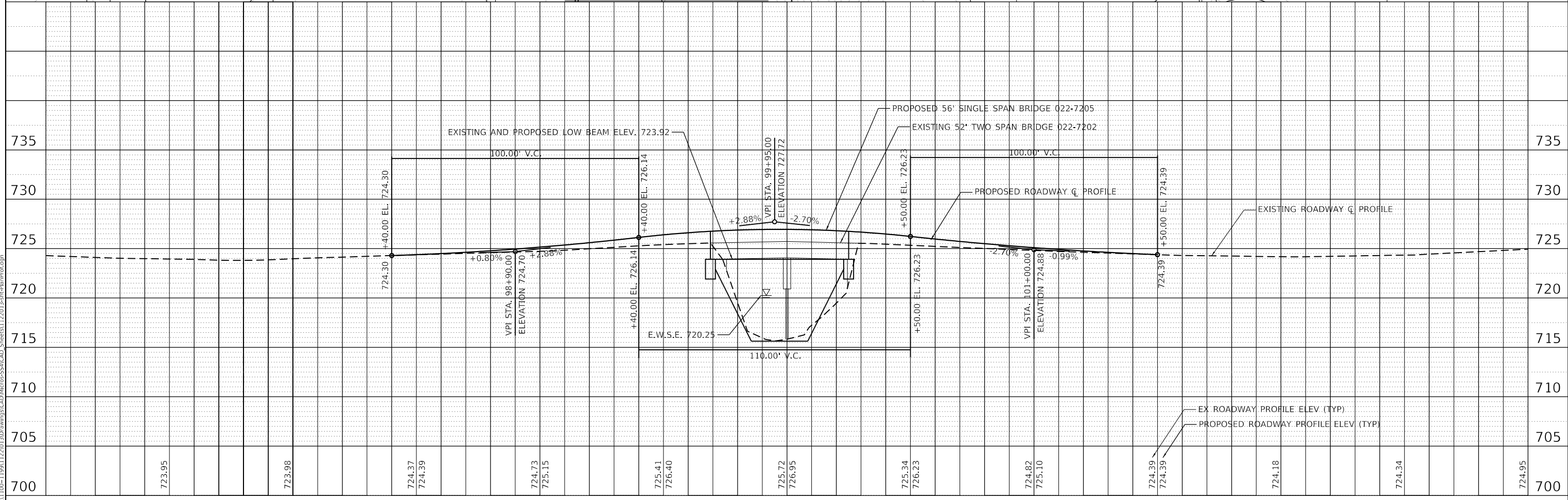
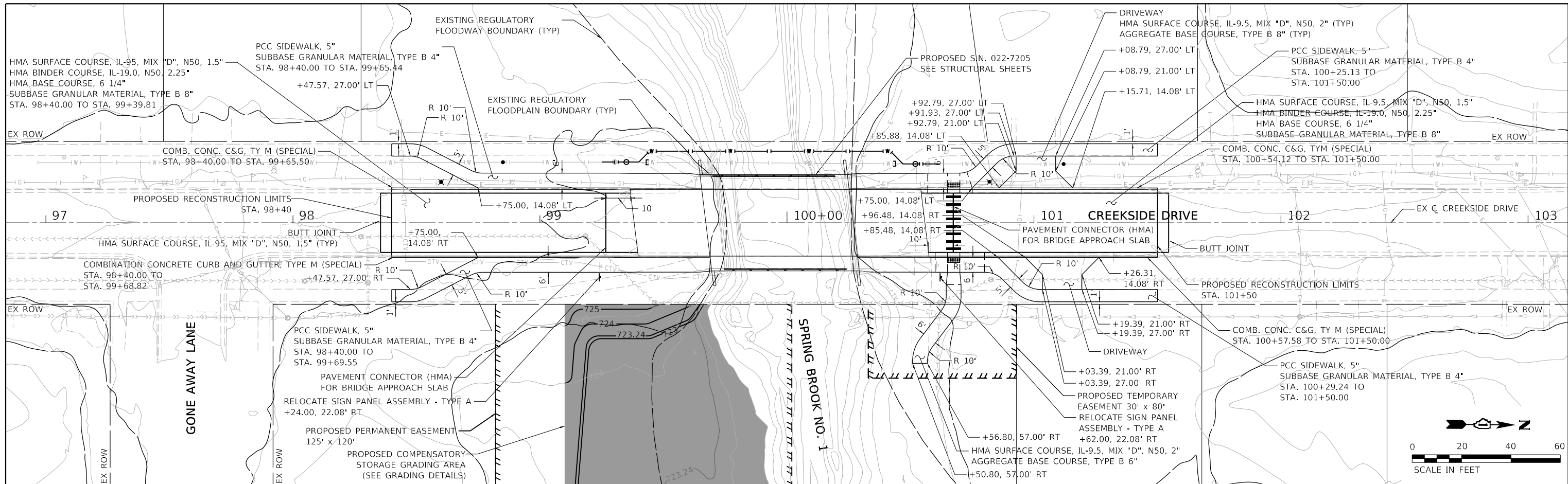
SCALE: 1"=20' SHEET 1 OF 1 SHEETS STA. 97+00 TO STA. 103+00

MS RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	8

DATE	
BY	
PLAN	
SURVEYED	
PLOTTED	
ALIGNMENT CHECKED	
NOTE BOOK NO.	
STRUCTURE NOTATION SHEET NO.	
CADD FILE NAME	

DATE	
BY	
PROFILE	
SURVEYED	
PLOTTED	
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NOTE BOOK NO.	
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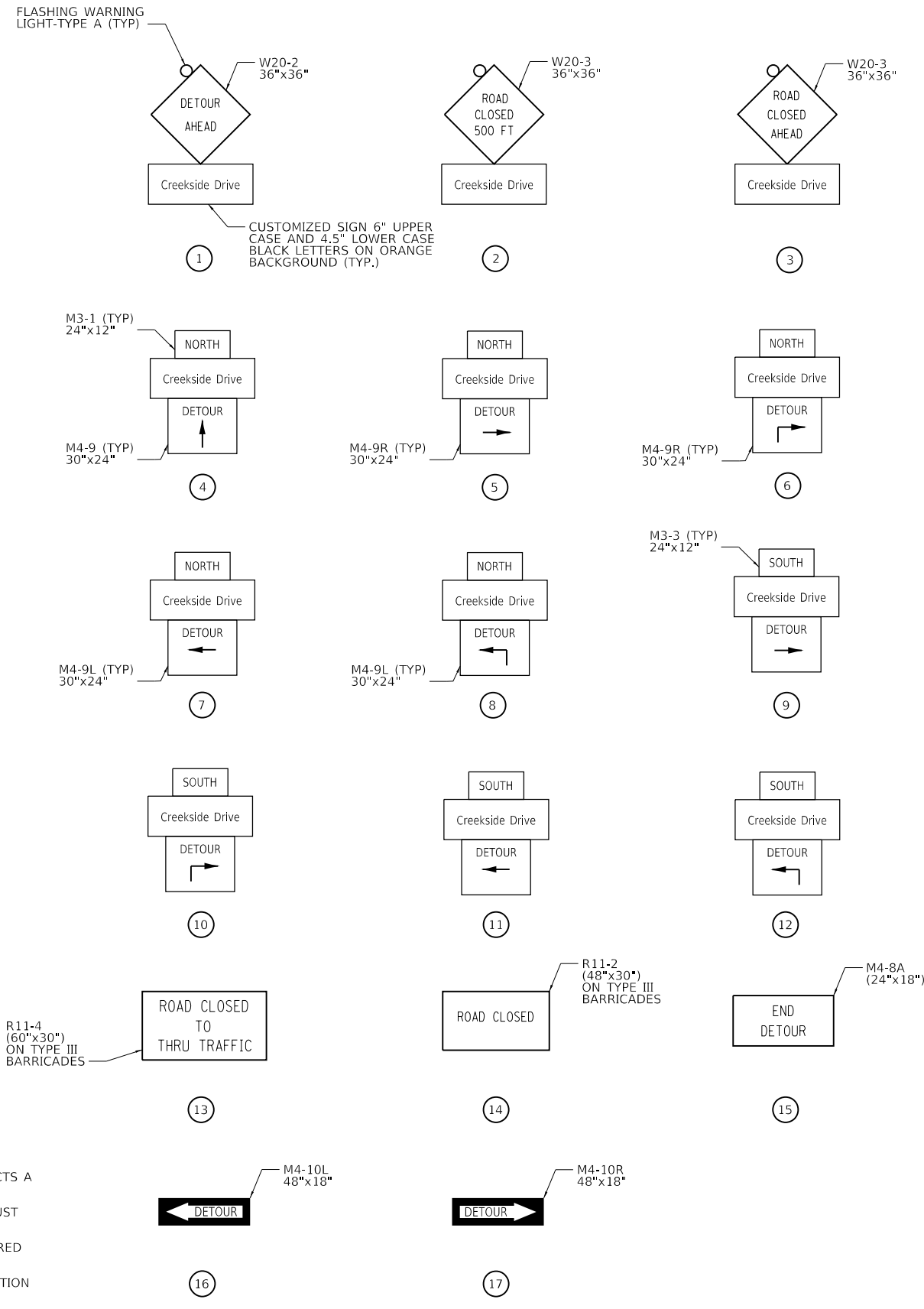
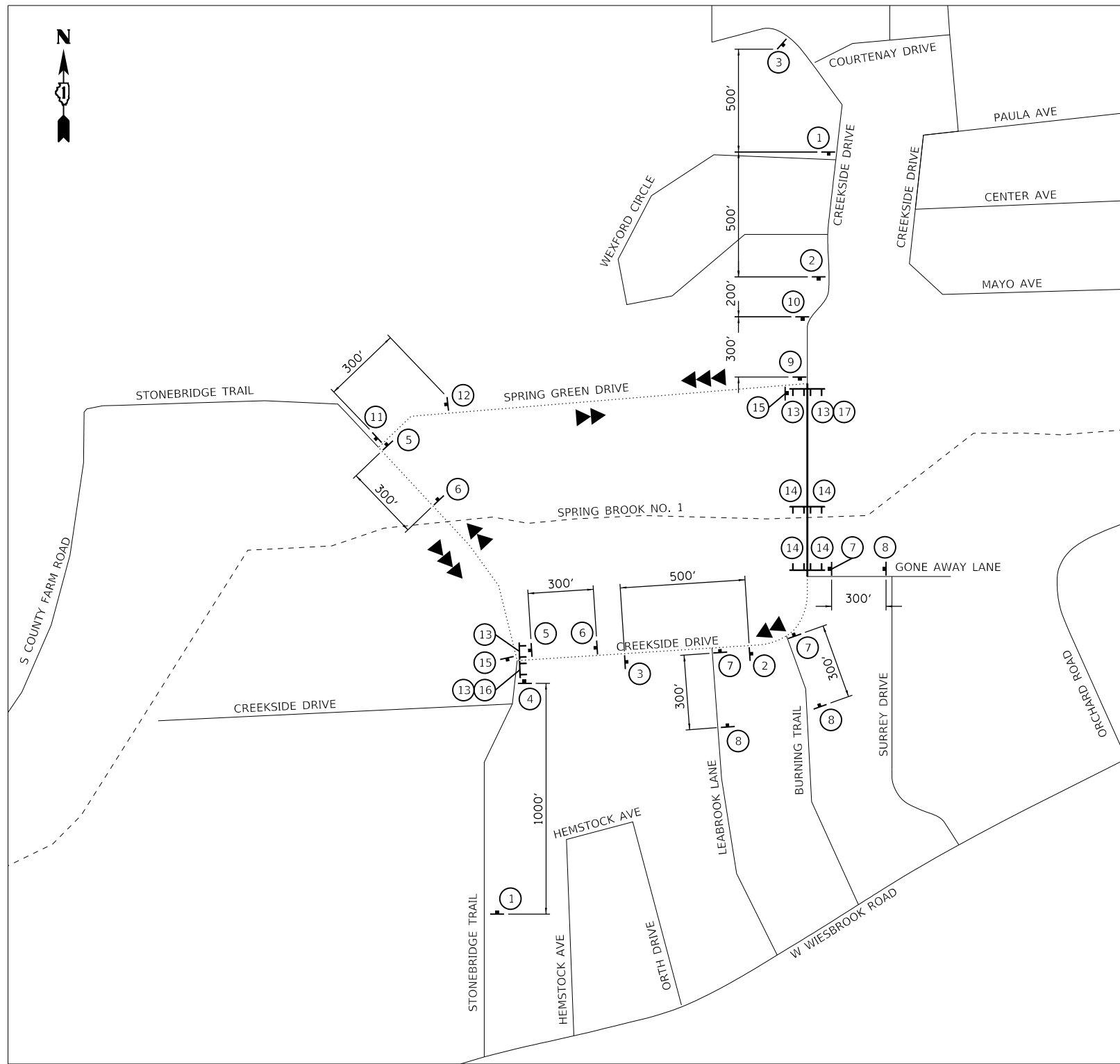


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	DATE -	REVISED -







**CITY OF WHEATON
DUPAGE COUNTY, ILLINOIS**

CREEKSIDE DRIVE OVER SPRING BROOK NO. 1 ROADWAY PLAN AND PROFILE			
SCALE: 1" = 20'	SHEET 1 OF 1 SHEETS	STA. 97+00.00 TO STA. 103+00.00	MS RTE. 4065

MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
CREEKSIDE DRIVE	DUPAGE	47	9



LEGEND

-  SIGN
-  TYPE III BARRICADES WITH TWO FLASHING LIGHTS
-  NORTHBOUND DETOURED TRAFFIC
-  SOUTHBOUND DETOURED TRAFFIC
-  PROPOSED DETOUR
-  ROAD CLOSED

DETOUR NOTES

1. THE CONTRACTOR SHALL CONTACT THE ENGINEER, PARK DISTRICT, EMERGENCY SERVICES, AND SCHOOL DISTRICTS A MINIMUM OF 72 HOURS PRIOR TO IMPLEMENTING THE DETOUR.
2. THE LOCATIONS OF ALL SIGNS SHOWN ON THE DETOUR PLAN ARE APPROXIMATE. THE CONTRACTOR SHALL ADJUST LOCATIONS TO FIT FIELD CONDITIONS.
3. EXISTING TRAFFIC SIGNS CONFLICTING WITH THE DETOUR SIGNAGE SHALL BE TEMPORARILY REMOVED OR COVERED AS DIRECTED BY THE ENGINEER.
4. ALL DETOUR SIGNS SHALL BE NEW OR IN LIKE-NEW CONDITION. THE ENGINEER SHALL DETERMINE IF THE CONDITION OF THE SIGNS IS SATISFACTORY.
5. DETOUR SIGNING SHALL HAVE BLACK LETTERS WITH 5" MIN. HEIGHT ON ORANGE BACKGROUND.
6. ALL DETOUR SIGNS SHALL HAVE THEIR OWN SIGN SUPPORTS.

**CITY OF WHEATON
DUPAGE COUNTY, ILLINOIS**

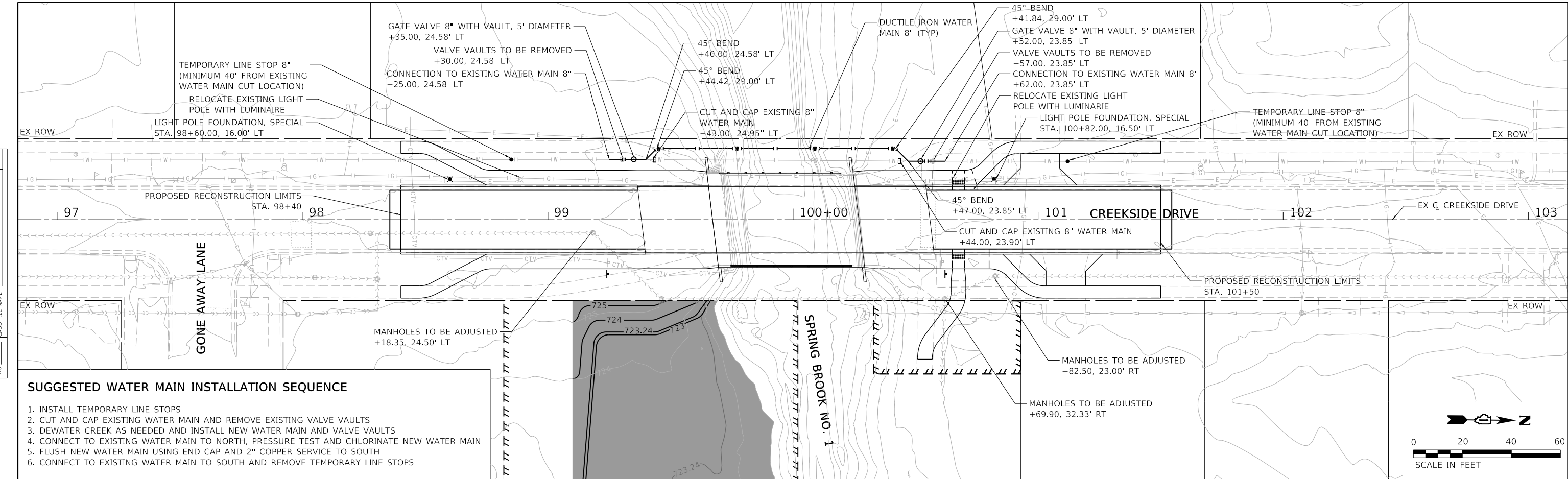
**CREEKSIDE DRIVE OVER SPRING BROOK NO. 1
DETOUR PLAN**

SCALE: NTS SHEET 1 OF 1 SHEETS STA. N/A TO STA. N/A

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4065	CREEKSIDE DRIVE	DUPAGE	47	10

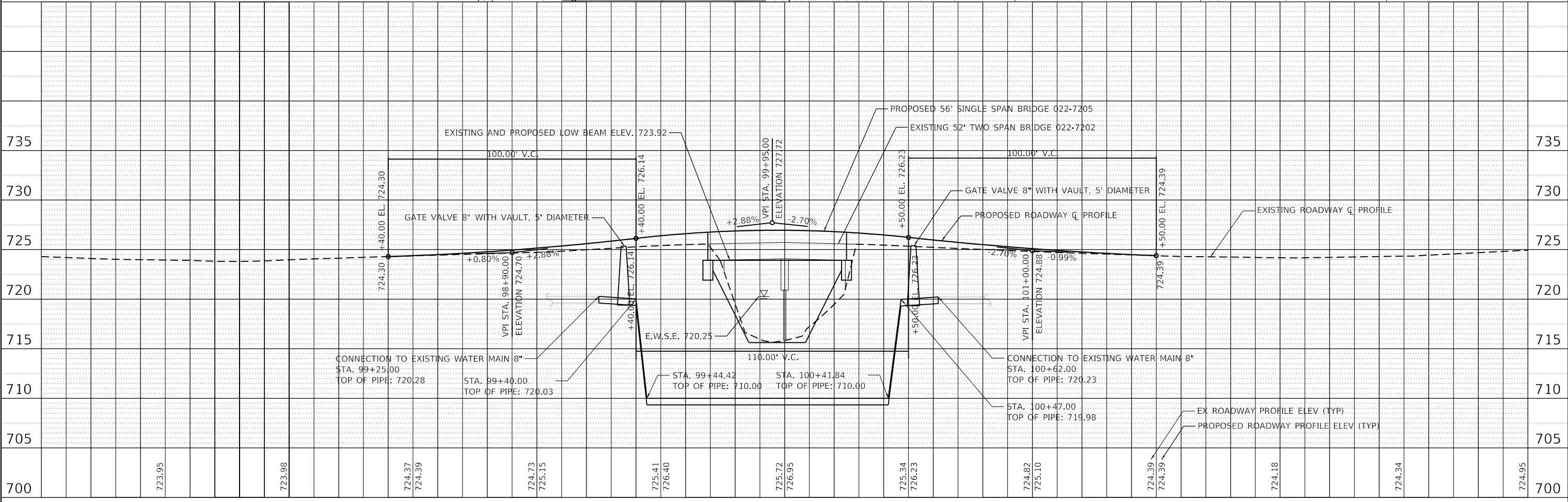
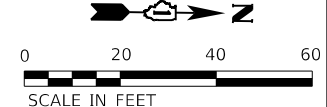
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NO.	
NO.	

DATE	
BY	
PROFILE	SURVEYED
	PLOTTED
	GRADES CHECKED
	STRUCTURE NOTATIONS CHECKED
NO.	
NO.	



SUGGESTED WATER MAIN INSTALLATION SEQUENCE

1. INSTALL TEMPORARY LINE STOPS
2. CUT AND CAP EXISTING WATER MAIN AND REMOVE EXISTING VALVE VAULTS
3. DEWATER CREEK AS NEEDED AND INSTALL NEW WATER MAIN AND VALVE VAULTS
4. CONNECT TO EXISTING WATER MAIN TO NORTH, PRESSURE TEST AND CHLORINATE NEW WATER MAIN
5. FLUSH NEW WATER MAIN USING END CAP AND 2" COPPER SERVICE TO SOUTH
6. CONNECT TO EXISTING WATER MAIN TO SOUTH AND REMOVE TEMPORARY LINE STOPS



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
**CITY OF WHEATON
DUPAGE COUNTY, ILLINOIS**


CREEKSIDE DRIVE OVER SPRING BROOK NO. 1 UTILITY PLAN AND PROFILE	
SCALE: 1" = 20'	SHEET 1 OF 1 SHEETS
STA. 97+00.00	TO STA. 103+00.00

MS RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	11

LEGEND

 SEEDING, CLASS 1B, AND EROSION CONTROL BLANKET *


 SEEDING, CLASS 2A, AND EROSION CONTROL BLANKET *

 SODDING, SALT TOLERANT *

 PERIMETER EROSION BARRIER

 INLET FILTERS

 TREE TRUNK PROTECTION

 DECIDUOUS TREE

* ALL DISTURBED AREAS SCHEDULED FOR PERMANENT SEEDING OR SODDING RESTORATION SHALL ALSO RECEIVE TEMPORARY EROSION CONTROL SEEDING AND TEMPORARY EROSION CONTROL BLANKET AS NEEDED DURING CONSTRUCTION PER SECTION 280 OF THE SSRBC.

* ALL PERMANENT SEEDING OR SODDING RESTORATION SHALL BE PLACED ON 4 INCHES OF NEW TOPSOIL IN ACCORDANCE WITH SECTION 211 OF THE SSRBC.

EROSION CONTROL AND LANDSCAPING GENERAL NOTES

1. ALL AREAS DISTURBED BY THE CONTRACTOR SHALL BE RESTORED WITH 4" TOPSOIL AND SEEDING OR SODDING AS SHOWN ON THE PLANS.
2. ALL TEMPORARY SOIL EROSION AND SEDIMENT CONTROL MEASURES SHALL BE REMOVED WITHIN THIRTY (30) DAYS AFTER FINAL SITE STABILIZATION IS ACHIEVED OR AFTER THE TEMPORARY MEASURES ARE NO LONGER NEEDED. TRAPPED SEDIMENT AND OTHER DISTURBED SOIL AREAS SHALL BE PERMANENTLY STABILIZED.
3. TEMPORARY EROSION CONTROL BLANKET SHALL BE USED ON ALL AREAS OF SOIL DISTURBANCES AND SHALL BE IN ACCORDANCE WITH THE ILLINOIS URBAN MANUAL, LATEST EDITION AND IDOT STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION LATEST EDITION.
4. EROSION CONTROL ITEMS ARE CONSIDERED TO BE HIGH PRIORITY ITEMS ON THIS CONTRACT. THE ENGINEER WILL IMPLEMENT ALL PROVISIONS OF THE SPECIFICATION NECESSARY TO ASSURE THAT EROSION CONTROL ITEMS ARE CONSTRUCTED AND MAINTAINED IN A TIMELY MANNER. ALL EROSION CONTROL MEASURES SHALL BE INSTALLED PRIOR TO BEGINNING ANY CONSTRUCTION ACTIVITIES WHICH WILL POTENTIALLY CREATE ERODIBLE CONDITIONS.
5. PERIMETER EROSION BARRIER IS SHOWN OUTSIDE THE RIGHT-OF-WAY FOR CLARITY. PERIMETER EROSION BARRIER SHOULD BE CONSTRUCTED 6" INSIDE THE RIGHT-OF-WAY IN THESE LOCATIONS.
6. CONTRACTOR SHALL SUBMIT AN IN-STREAM WORK PLAN TO DUPAGE COUNTY SOIL & WATER CONSERVATION DISTRICT FOR APPROVAL PRIOR TO WORK WITHIN THE WATERWAY.

ALL LOCATIONS SHOWN FOR PROPOSED LANDSCAPING MATERIALS ARE APPROXIMATE AND SHALL BE APPROVED BY ENGINEER IN FIELD PRIOR TO INSTALLATION.

ANTICIPATED CONSTRUCTION SEQUENCE

1. INSTALL TREE TRUNK PROTECTION AND TEMPORARY EROSION AND SEDIMENT CONTROL SUCH AS PERIMETER EROSION BARRIER.
2. PERFORM TREE REMOVAL, DEMOLITION, AND EARTHWORK NECESSARY FOR ROADWAY AND BRIDGE IMPROVEMENTS.
3. PROVIDE TEMPORARY SEEDING AND TEMPORARY EROSION CONTROL BLANKET ON ALL DISTURBED AREAS.
4. PERFORM TEMPORARY BYPASS AND DEWATERING OPERATIONS FOR IN-STREAM WORK AS NEEDED ACCORDING TO CONTRACTOR'S APPROVED IN-STREAM WORK PLAN.
5. INSTALL AND/OR RELOCATE ALL UNDERGROUND UTILITIES.
6. INSTALL REPLACEMENT STRUCTURE.
7. AFTER FINAL CONSTRUCTION OF ALL ROADWAY ITEMS, PERFORM FINAL LANDSCAPING AND EROSION CONTROL INSTALLATION.

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**CITY OF WHEATON
DUPAGE COUNTY, ILLINOIS**

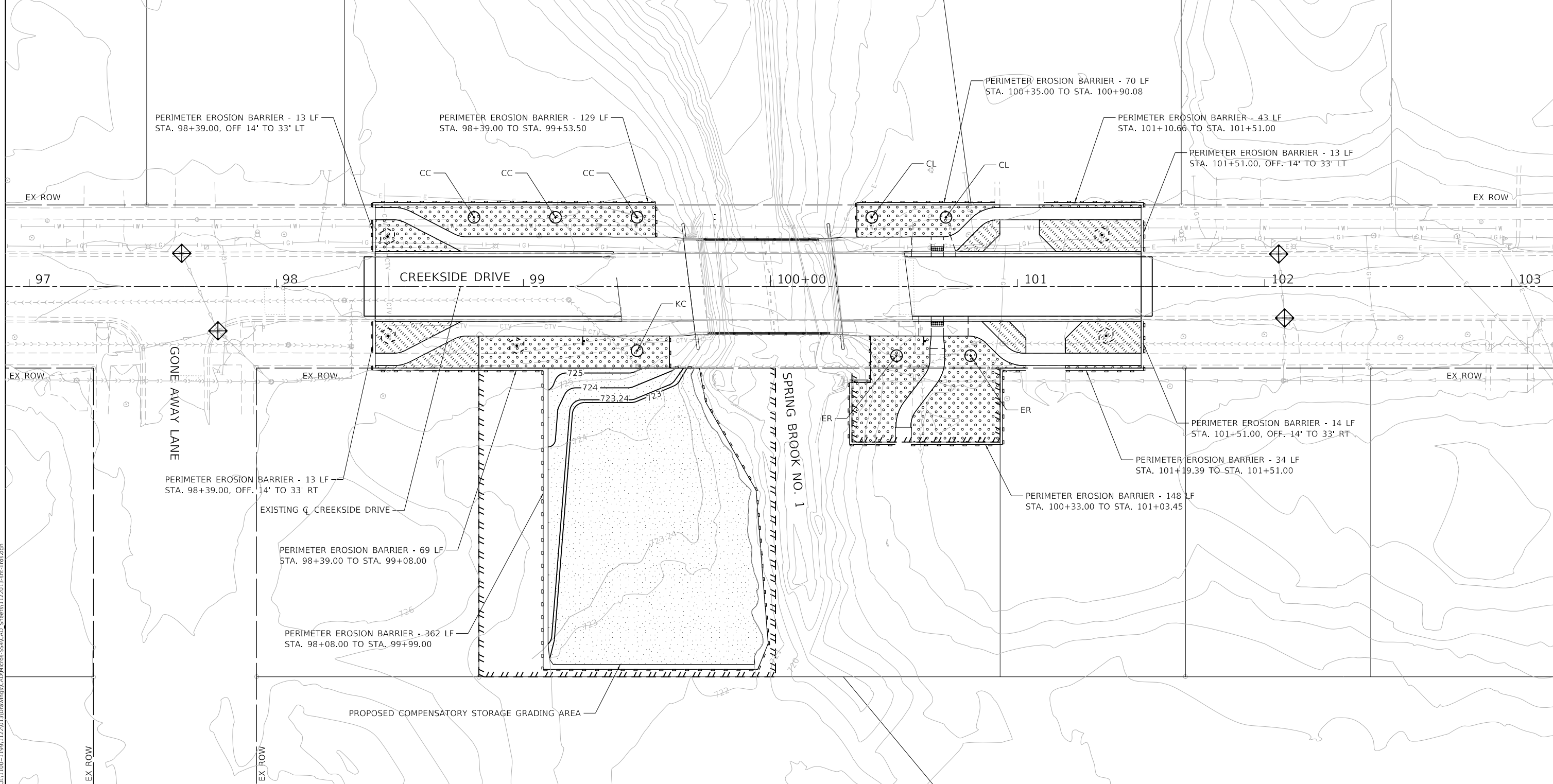
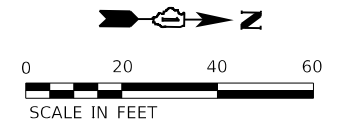
**CREEKSIDE DRIVE OVER SPRING BROOK NO. 1
LANDSCAPING AND EROSION CONTROL NOTES**

SCALE: NTS SHEET 1 OF 2 SHEETS STA. N/A TO STA. N/A

MS RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	12

PLANT DATA CHART

PLANT CODE	COMMON NAME	SCIENTIFIC NAME	AVG. MATURE HEIGHT	AVG. MATURE SPREAD	SIZE WHEN PLANTED	ROOT ZONE MODE	MINIMUM SIZE				MULCH RING DIA.	PLANT QUANTITY
							MIN. BALL OR POT SIZE	MIN. HOLE SIZE				
							MIN. CONT. SIZE	MIN. CONT. DEPTH	MIN. HOLE DIA.	MIN. HOLE DEPTH	TOTAL	
LARGE GROWING DECIDUOUS TREES												
KC	Kentucky Coffee Tree	Gymnocladus dioicus	65'	40'	4" Cal.	B&B	42"	25"	58"	25"	48"	1
CL	Crimean Linden	Tilia euchlorea	40'	25'	3" Cal.	B&B	32"	19"	48"	19"	48"	2
MEDIUM GROWING DECIDUOUS TREES												
CC	American Hornbeam	Carpinus caroliniana	25'	25'	3" Cal.	B&B	32"	19"	48"	19"	48"	3
ER	Eastern Redbud	Cercis canadensis	30'	25-35'	3" Cal.	B&B	32"	19"	48"	19"	48"	2



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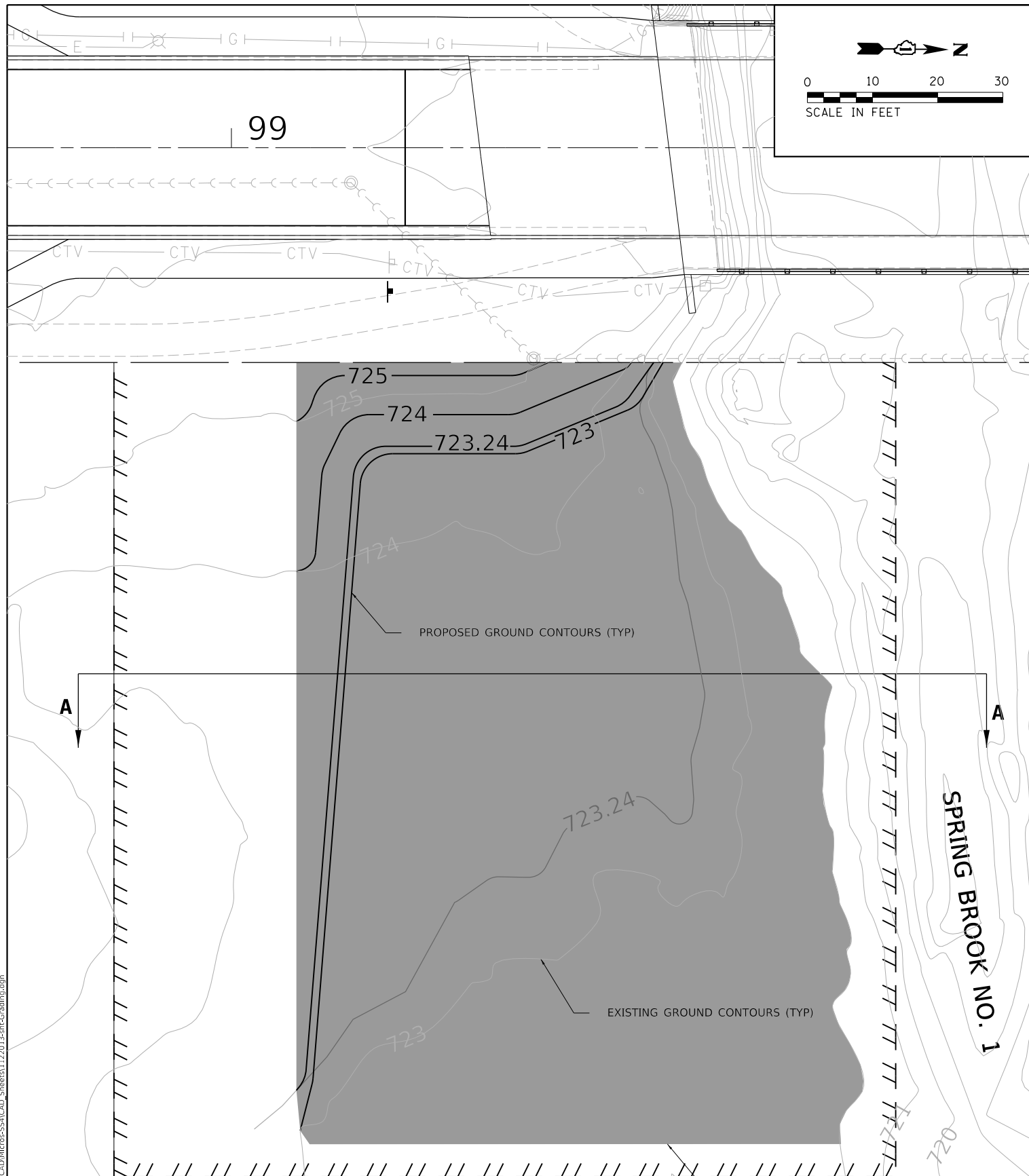
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	DATE -	REVISED -

**CITY OF WHEATON
DUPAGE COUNTY, ILLINOIS**

**CREEKSIDE DRIVE OVER SPRING BROOK NO. 1
LANDSCAPING AND EROSION CONTROL PLAN**

SCALE: 1"=20" SHEET 2 OF 2 SHEETS STA. 97+00 TO STA. 103+00

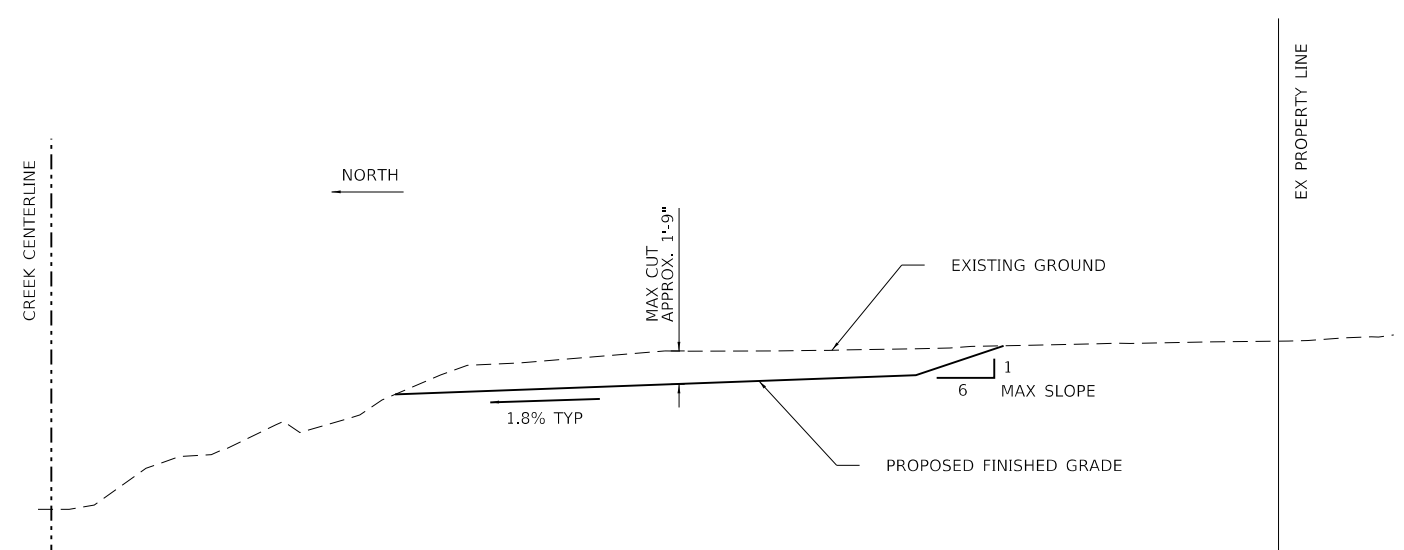
MS RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	13



PROPOSED PERMANENT EASEMENT 125' x 120'

PLAN

PROPOSED COMPENSATORY STORAGE GRADING AREA



SECTION A-A
 HORIZONTAL SCALE: 1" = 10'
 VERTICAL SCALE: 1" = 5'

**CITY OF WHEATON
 DUPAGE COUNTY, ILLINOIS**

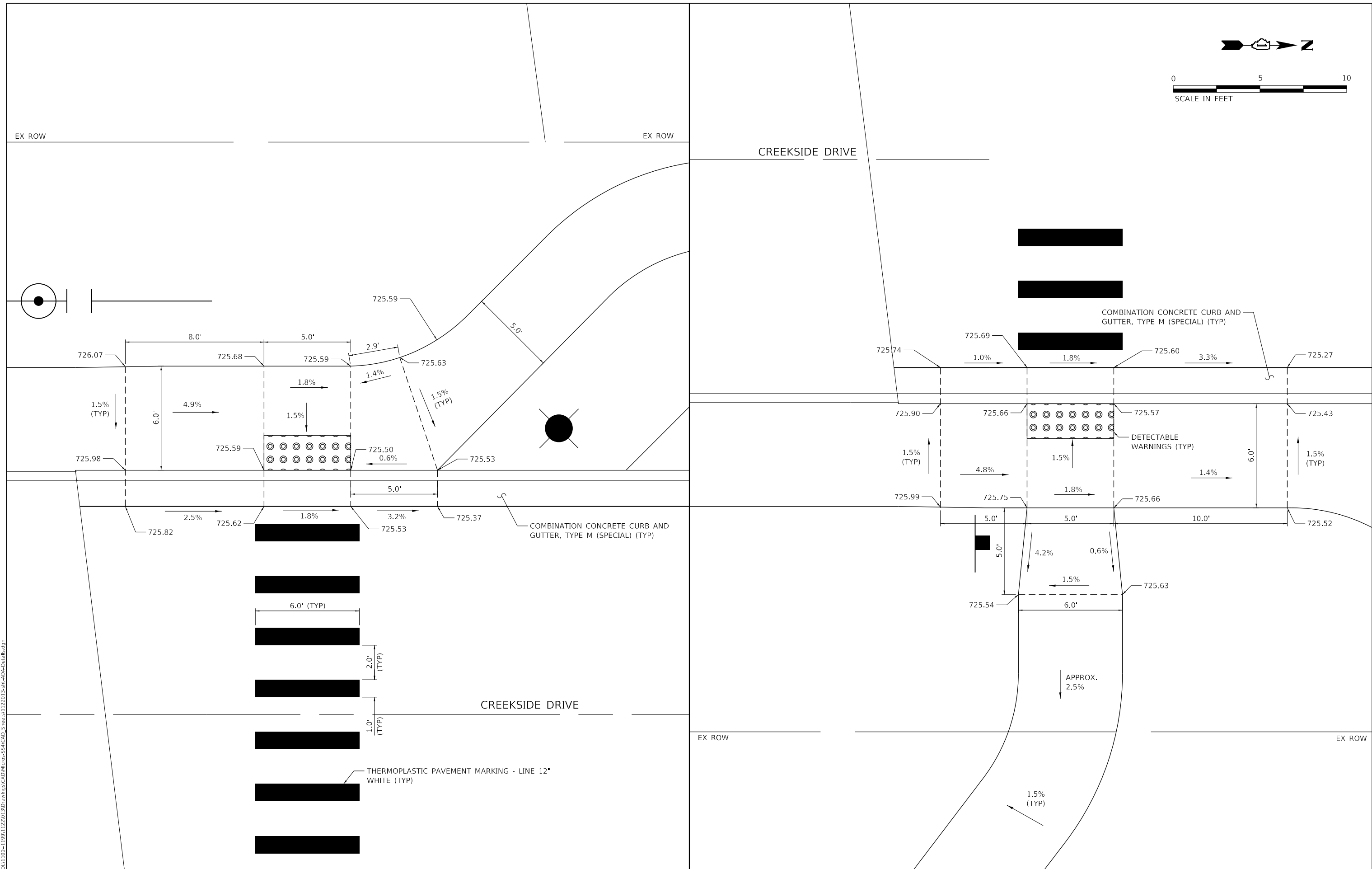
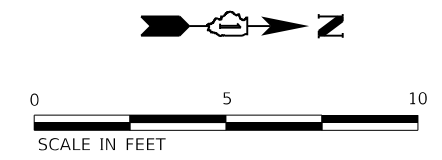
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 COMPENSATORY STORAGE GRADING PLAN**

SCALE: SHEET OF SHEETS STA. 97+00 TO STA. 103+00

MS RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	14

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	DATE -	REVISED -

CITY OF WHEATON
DUPAGE COUNTY, ILLINOIS

CREEKSIDE DRIVE OVER SPRING BROOK NO. 1
CURB RAMP DETAILS

SCALE: 1"=5' SHEET 1 OF 1 SHEETS STA. N/A TO STA. N/A

MS RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	15

Benchmark: BM #3: Set cut square on top of concrete headwall at the NW corner of Creekside Drive bridge over Spring Brook No. 1, Elev. 729.589
 Existing Structure: Structure Number 022-7202, Creekside Drive, Station 100+00.00 was built in 1970 as the Creekside Drive Bridge over Spring Brook in DuPage County. The superstructure consists of a two-span, 15-inch-deep concrete slab. The structure has a total span of 52'-0" and an out-to-out deck width of 39'-6". The concrete slab superstructure is supported on monolithically-poured reinforced concrete abutment and pier caps on timber piles.
 Traffic Control: Detour
 Salvage: N/A

SCOPE OF WORK

1. Relocate existing buried and structure-attached utilities.
2. Remove existing structure.
3. Construct cast-in-place concrete foundations on driven HP Piles for proposed bridge crossing.
4. Erect prefabricated galvanized steel tub girders.
5. Construct cast-in-place concrete bridge deck, sidewalks, and parapets.
6. Construct approach slabs for roadway.

GENERAL NOTES

Reinforcement bars designated (E) shall be epoxy coated.
 Layout of the slope protection system may be varied to suit ground conditions in the field as directed by the Engineer.
 The embankment configuration shown shall be the minimum that must be placed and compacted prior to construction of the abutments.

LOADING HL-93

Allow 50 psf for future wearing surface

SEISMIC DATA

Seismic Performance Zone (SPZ) = 1
 Design Spectral Acceleration at 1.0 sec. (SD1) = 0.103g
 Design Spectral Acceleration at 0.2 sec. (SDS) = 0.164g
 Soil Site Class = D

DESIGN SPECIFICATIONS

2020 AASHTO LRFD Bridge Design Specifications
 Customary U.S. Units, 9th Edition

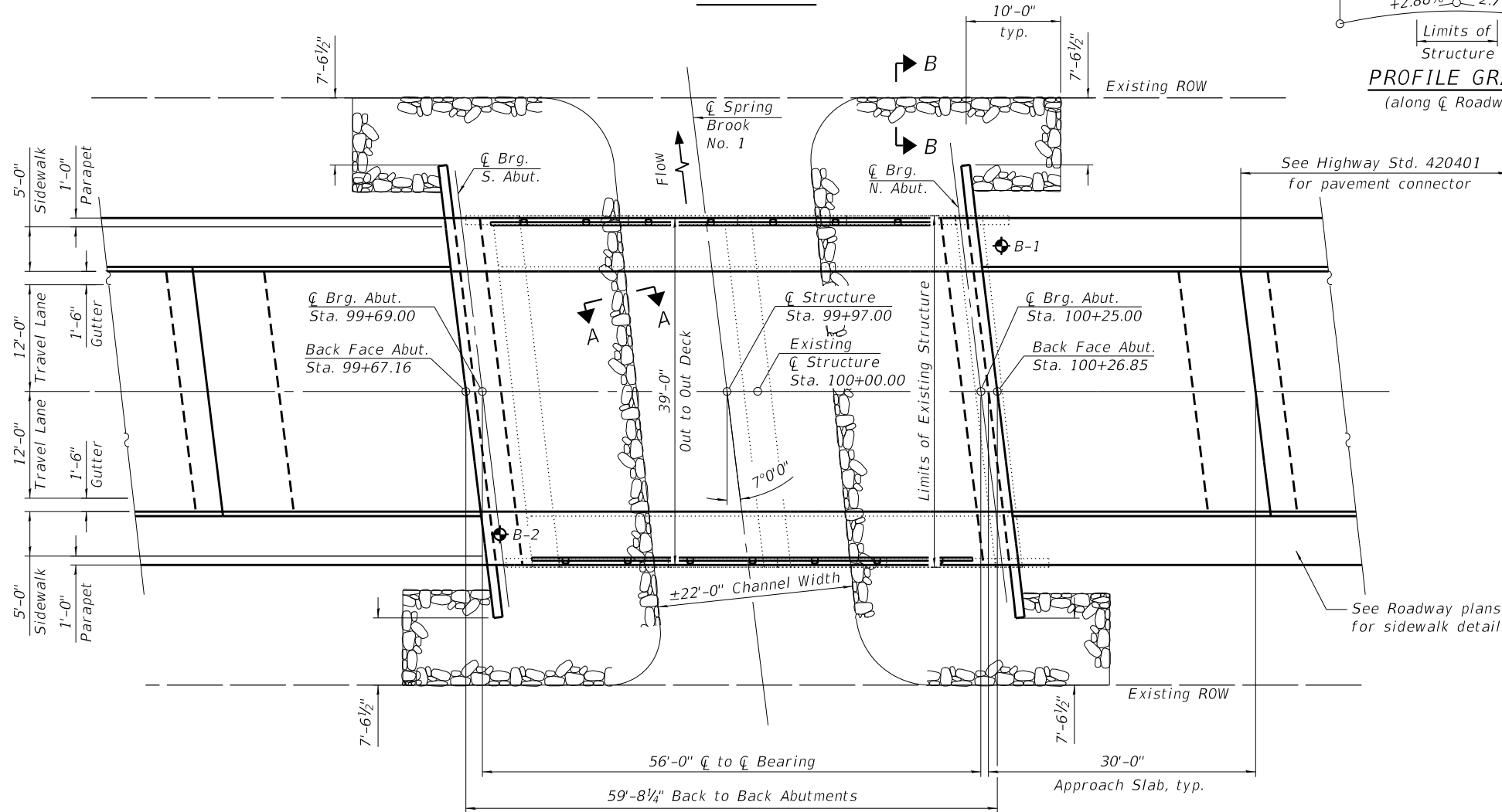
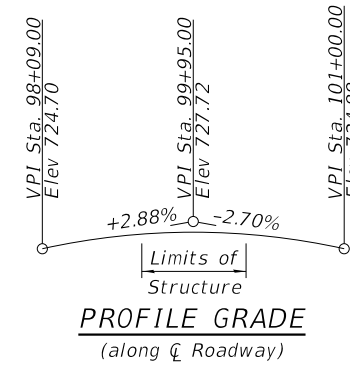
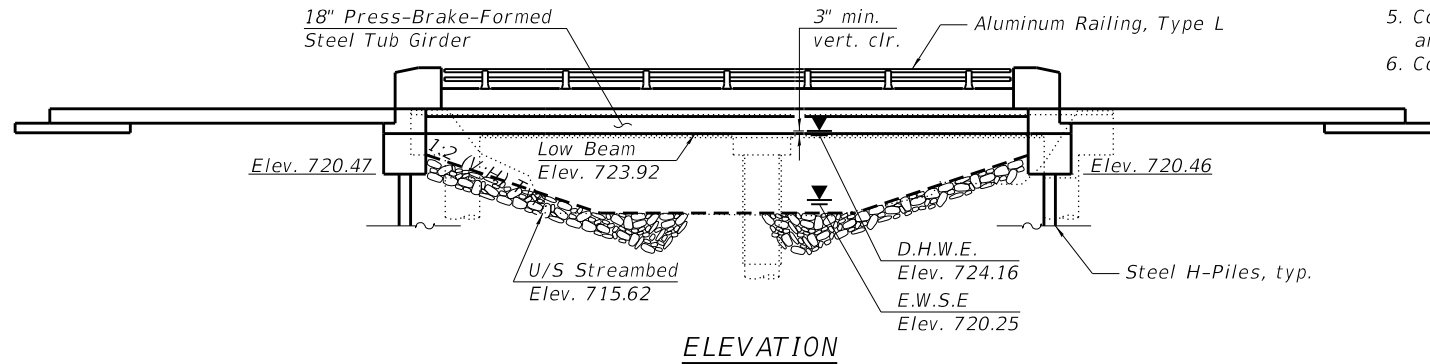
DESIGN STRESSES

FIELD UNITS

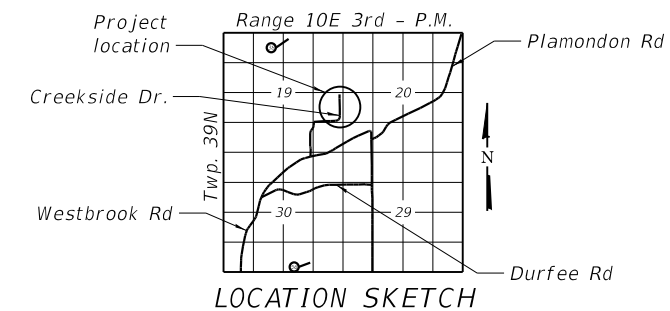
$f'_c = 3,500$ psi (Substructure)
 $f_y = 4,000$ psi (Superstructure)
 $f_y = 60,000$ psi (Reinforcement)

PREFABRICATED UNITS

$f_y = 50,000$ psi (M270 Grade 50, Primary Members)



I certify that to the best of knowledge, information and belief, this substructure design is structurally adequate for the design loading shown on the plans. The design is an economical one for the style of structure and complies with requirements of the current AASHTO LRFD Specifications.



WATERWAY INFORMATION

Drainage Area = 4.28 SQ MI Low Grade Elev. 723.17 @ Sta. 97+72

Flood	Freq. Yr.	Q C.F.S.	Opening Ft ²		Head - Ft.		Headwater El.		
			Exist.	Prop.	H.W.E. Exist.	Prop.	Exist.	Prop.	
Ten-Year	10	658.00	279	325	723.17	0.02	0.00	723.19	723.16
Design	30	1061.30	279	325	724.16	0.12	0.02	724.28	724.18
Base	100	1375.50	279	325	725.36	0.08	0.03	725.44	725.39
Overtopping	30	1061.30	279	325	724.16	0.12	0.02	724.28	724.18
Max. Calc.	500	2033.30	279	325	726.64	0.02	0.02	726.66	726.66

DESIGN SCOUR ELEVATION TABLE

Event / Limit	Design Scour Elevations (ft.)		
	State	S. Abut.	N. Abut.
Q100	720.47	720.46	720.46
Q200	720.47	720.46	720.46
Design	720.47	720.46	720.46
Check	720.47	720.46	720.46

**GENERAL PLAN & ELEVATION
 CREEKSIDE DRIVE OVER
 SPRING BROOK NO. 1
 DUPAGE COUNTY
 STATION 99+97.00
 STRUCTURE NO. 022-7205**

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1170 SOUTH HOUBOLT ROAD
 JOLIET, ILLINOIS 60431
 (815) 744-4200
 IDFPR NO. 184-001273

USER NAME = _____
 DESIGNED - MJD
 CHECKED - AJS
 PLOT SCALE = _____
 DRAWN - CJH
 PLOT DATE = _____
 CHECKED - _____

REVISED - _____
 REVISED - _____
 REVISED - _____
 REVISED - _____

**CITY OF WHEATON
 DUPAGE COUNTY, ILLINOIS**

**GENERAL PLAN AND ELEVATION
 STRUCTURE NO. 022-7205**

SHEET 1 OF 20 SHEETS

M.S. RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	16

GENERAL NOTES

Fasteners shall be ASTM A325 Type 1, mechanically galvanized bolts (in painted areas and ASTM A325 Type 3 in unpainted areas). Bolts 1 in Φ , holes 1½ in Φ , unless otherwise noted

Calculated weight of Structural Steel = _____ lbs. (M270 Grade 50)***
 _____ lbs. (A572 Grade 65)***

All structural steel and primary members shall be AASHTO M270 Grade 50.

No field welding is permitted except as specified in the contract documents.

Reinforcement bars designated (E) shall be epoxy coated.

Layout of the slope protection system may be varied to suit ground conditions in the field as directed by the Engineer.

The embankment configuration shown shall be the minimum that must be placed and compacted prior to construction of the abutments.

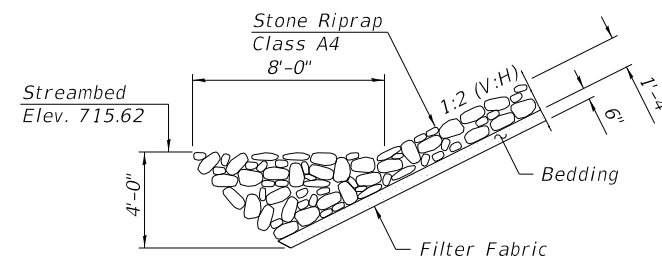
The Contractor shall obtain a construction permit from the Illinois Department of Natural Resources for any temporary construction activity placed in the water expect cofferdams. This shall include the placement of material for run-arounds, causeways, etc. Any permit application by the Contractor shall refer to the IDNR 3704 Floodway Construction permit number allowing permanent construction as shown in the contract plans.

Plans are for a Press-Brake-Formed Steel Tub Girder (PBFSTG) superstructure. The provided details and layout are for the general design and layout and may be modified as required for the actual prefabricated bridge system that is used. All adjustments shall be submitted to the Engineer for review & approval and will not be cause for additional compensation for a change in scope of the work. However, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

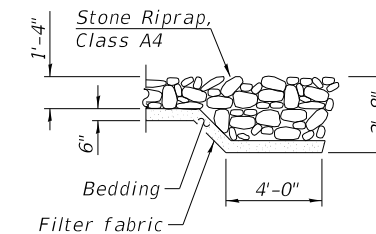
*** Information to be provided by the PBFSTG manufacturer. See Special Provisions.

INDEX OF SHEETS

1. General Plan
2. General Data
3. Top of Deck Elevations
4. Top of Deck Elevations Tables
5. Top of South Approach Slab Elevations
6. Top of North Approach Slab Elevations
7. Superstructure
8. Superstructure Details
9. Integral Abutment Diaphragm Details
10. Bridge Approach Slab Details (1 of 2)
11. Bridge Approach Slab Details (2 of 2)
12. Railing Layout
13. Aluminum Railing, Type L
14. Structural Steel
15. Structural Steel Details
16. South Abutment
17. North Abutment
18. HP Pile Details
19. Soil Borings (1 of 2)
20. Soil Borings (2 of 2)



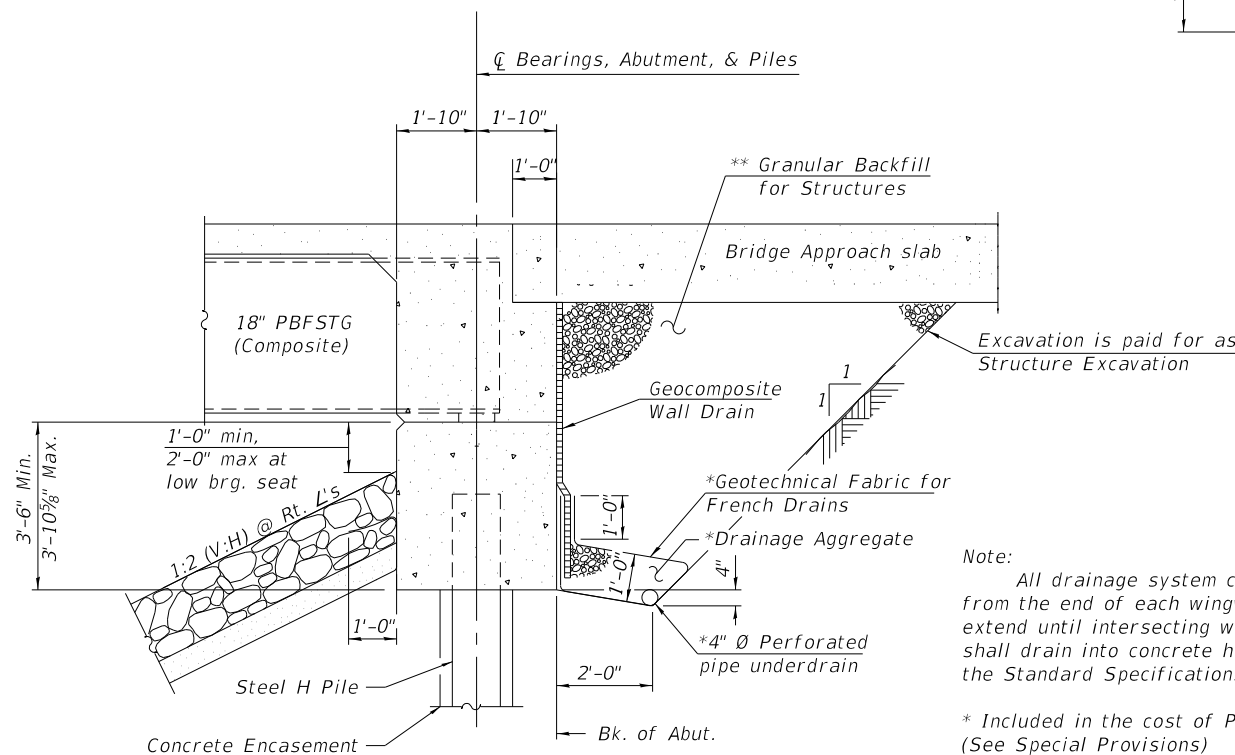
SECTION A-A



SECTION B-B

CREEKSIDE DRIVE OVER
 SPRING BROOK NO.1
 BUILT 2023 BY
 CITY OF WHEATON
 STATION 99+97.00
 STR NO. 022-7205
 HL-93 LOADING

NAME PLATE
 See Std. 515001



SECTION THRU ABUTMENT
 (Dimensions are @ Rt. L's)

Note:
 All drainage system components shall extend to 2'-0" from the end of each wingwall except an outlet pipe shall extend until intersecting with the side slopes. The pipes shall drain into concrete headwalls. (See Article 601.50 of the Standard Specifications and Highway Standard 601101).

* Included in the cost of Pipe Underdrains for Structures. (See Special Provisions)

** Granular Backfill for Structures shall be placed and compacted according to section 502.10 of the Standard Specifications.

TOTAL BILL OF MATERIAL

Item	Unit	Super	Sub	Total
Stone Riprap, Class A4	Sq. Yd.		460	460
Filter Fabric	Sq. Yd.		460	460
Protective Coat	Sq. Yd.	469		469
Removal of Existing Structures	Each		1	1
Structure Excavation	Cu. Yd.		135	135
Concrete Structures	Cu. Yd.		62.2	62.2
Concrete Superstructure (Approach Slab)	Cu. Yd.	77.8		77.8
Concrete Superstructure	Cu. Yd.	70.2		70.2
Bridge Deck Grooving	Sq. Yd.	327		327
Concrete Encasement	Cu. Yd.		5	5
Reinforcement Bars, Epoxy Coated	Pound	48,577	8,948	57,525
Aluminum Railing, Type L	Foot	100		100
Furnishing Steel Piles HP10X42	Foot		520	520
Driving Piles	Foot		520	520
Test Pile Steel HP 10X42	Each		2	2
Piles Shoes	Each		12	12
Name Plates	Each		1	1
Anchor Bolts, 1"	Each	28		28
Granular Backfill for Structures	Cu. Yd.		28	28
Geocomposite Wall Drain	Sq. Yd.		54	54
Pipe Underdrains for Structures 4"	Foot		103	103
Press-Brake-Formed Steel Tub Girder (PBFSTG)	Pound	41,619		41,619

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SA STRAND ASSOCIATES
 1170 SOUTH HOUBOLT ROAD
 JOLIET, ILLINOIS 60431
 (815) 744-4200
 IDFPR NO. 184-001273

USER NAME =	DESIGNED - MJD	REVISED -
PLOT SCALE =	CHECKED - AJS	REVISED -
PLOT DATE =	DRAWN - CJH	REVISED -
	CHECKED -	REVISED -

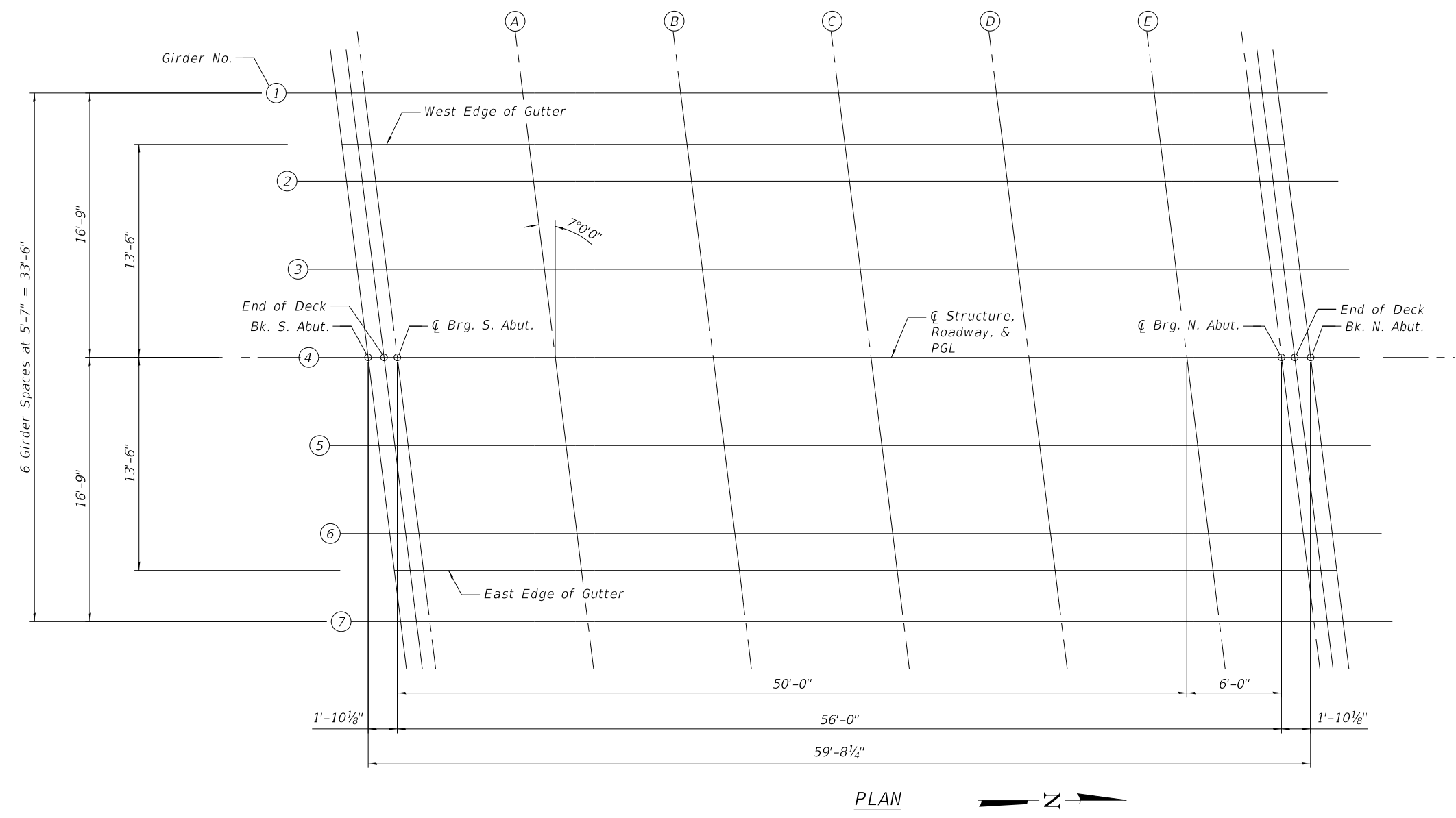
**CITY OF WHEATON
 DUPAGE COUNTY, ILLINOIS**

**GENERAL DATA
 STRUCTURE NO. 022-7205**

SHEET 2 OF 20 SHEETS

M.S. RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	17

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PLAN

For Girder information, see sheet 14 of 20.



1170 SOUTH HOUBOLT ROAD
 JULIET, ILLINOIS 60431
 (815) 744-4200
 IDFP NO. 184-001273

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PLOT DATE =	CHECKED -	REVISED -

CITY OF WHEATON
 DUPAGE COUNTY, ILLINOIS

TOP OF DECK ELEVATIONS
 STRUCTURE NO. 022-7205

SHEET 3 OF 20 SHEETS

M.S. RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	18

BEAM 1

Location	Station	Offset	Theoretical Grade Elevations	*Theoretical Grade Elevations Adjusted for Dead Load Deflection
Back of S. Abut.	99+65.09	-16.75	726.37	
S. End of Deck	99+66.10	-16.75	726.38	
CL Brg. S. Abut.	99+66.94	-16.75	726.40	
A	99+76.94	-16.75	726.52	
B	99+86.94	-16.75	726.60	
C	99+96.94	-16.75	726.62	
D	100+06.94	-16.75	726.60	
E	100+16.94	-16.75	726.52	
CL Brg. N. Abut.	100+22.94	-16.75	726.45	
N. End of Deck	100+23.78	-16.75	726.44	
Back of N. Abut.	100+24.79	-16.75	726.42	

BEAM 3

Location	Station	Offset	Theoretical Grade Elevations	*Theoretical Grade Elevations Adjusted for Dead Load Deflection
Back of S. Abut.	99+66.46	-5.58	726.61	
S. End of Deck	99+67.47	-5.58	726.63	
CL Brg. S. Abut.	99+68.31	-5.58	726.64	
A	99+78.31	-5.58	726.76	
B	99+88.31	-5.58	726.83	
C	99+98.31	-5.58	726.85	
D	100+08.31	-5.58	726.81	
E	100+18.31	-5.58	726.73	
CL Brg. N. Abut.	100+24.31	-5.58	726.65	
N. End of Deck	100+25.15	-5.58	726.64	
Back of N. Abut.	100+26.16	-5.58	726.63	

BEAM 6

Location	Station	Offset	Theoretical Grade Elevations	*Theoretical Grade Elevations Adjusted for Dead Load Deflection
Back of S. Abut.	99+68.52	11.17	726.53	
S. End of Deck	99+69.53	11.17	726.55	
CL Brg. S. Abut.	99+70.37	11.17	726.56	
A	99+80.37	11.17	726.67	
B	99+90.37	11.17	726.72	
C	100+00.37	11.17	726.73	
D	100+10.37	11.17	726.69	
E	100+20.37	11.17	726.59	
CL Brg. N. Abut.	100+26.37	11.17	726.51	
N. End of Deck	100+27.21	11.17	726.50	
Back of N. Abut.	100+28.22	11.17	726.48	

WEST EDGE OF GUTTER

Location	Station	Offset	Theoretical Grade Elevations	*Theoretical Grade Elevations Adjusted for Dead Load Deflection
Back of S. Abut.	99+65.49	-13.50	726.44	
S. End of Deck	99+66.50	-13.50	726.46	
CL Brg. S. Abut.	99+67.34	-13.50	726.47	
A	99+77.34	-13.50	726.59	
B	99+87.34	-13.50	726.67	
C	99+97.34	-13.50	726.69	
D	100+07.34	-13.50	726.66	
E	100+17.34	-13.50	726.58	
CL Brg. N. Abut.	100+23.34	-13.50	726.51	
N. End of Deck	100+24.18	-13.50	726.50	
Back of N. Abut.	100+25.19	-13.50	726.48	

☐ STRUCTURE, ☐ ROADWAY & PGL/BEAM 4

Location	Station	Offset	Theoretical Grade Elevations	*Theoretical Grade Elevations Adjusted for Dead Load Deflection
Back of S. Abut.	99+67.15	0.00	726.74	
S. End of Deck	99+68.16	0.00	726.75	
CL Brg. S. Abut.	99+69.00	0.00	726.76	
A	99+79.00	0.00	726.88	
B	99+89.00	0.00	726.94	
C	99+99.00	0.00	726.96	
D	100+09.00	0.00	726.92	
E	100+19.00	0.00	726.83	
CL Brg. N. Abut.	100+25.00	0.00	726.76	
N. End of Deck	100+25.84	0.00	726.74	
Back of N. Abut.	100+26.84	0.00	726.73	

EAST EDGE OF GUTTER

Location	Station	Offset	Theoretical Grade Elevations	*Theoretical Grade Elevations Adjusted for Dead Load Deflection
Back of S. Abut.	99+68.81	13.50	726.49	
S. End of Deck	99+69.82	13.50	726.50	
CL Brg. S. Abut.	99+70.65	13.50	726.51	
A	99+80.65	13.50	726.62	
B	99+90.65	13.50	726.68	
C	100+00.65	13.50	726.68	
D	100+10.65	13.50	726.64	
E	100+20.65	13.50	726.54	
CL Brg. N. Abut.	100+26.65	13.50	726.46	
N. End of Deck	100+27.49	13.50	726.45	
Back of N. Abut.	100+28.50	13.50	726.43	

BEAM 2

Location	Station	Offset	Theoretical Grade Elevations	*Theoretical Grade Elevations Adjusted for Dead Load Deflection
Back of S. Abut.	99+65.78	-11.17	726.49	
S. End of Deck	99+66.79	-11.17	726.51	
CL Brg. S. Abut.	99+67.63	-11.17	726.52	
A	99+77.63	-11.17	726.64	
B	99+87.63	-11.17	726.71	
C	99+97.63	-11.17	726.73	
D	100+07.63	-11.17	726.70	
E	100+17.63	-11.17	726.62	
CL Brg. N. Abut.	100+23.63	-11.17	726.55	
N. End of Deck	100+24.47	-11.17	726.54	
Back of N. Abut.	100+25.47	-11.17	726.53	

BEAM 5

Location	Station	Offset	Theoretical Grade Elevations	*Theoretical Grade Elevations Adjusted for Dead Load Deflection
Back of S. Abut.	99+67.84	5.58	726.63	
S. End of Deck	99+68.84	5.58	726.65	
CL Brg. S. Abut.	99+69.68	5.58	726.66	
A	99+79.68	5.58	726.77	
B	99+89.68	5.58	726.83	
C	99+99.68	5.58	726.84	
D	100+09.68	5.58	726.80	
E	100+19.68	5.58	726.71	
CL Brg. N. Abut.	100+25.68	5.58	726.63	
N. End of Deck	100+26.52	5.58	726.62	
Back of N. Abut.	100+27.53	5.58	726.61	

BEAM 7

Location	Station	Offset	Theoretical Grade Elevations	*Theoretical Grade Elevations Adjusted for Dead Load Deflection
Back of S. Abut.	99+69.21	16.75	726.43	
S. End of Deck	99+70.21	16.75	726.44	
CL Brg. S. Abut.	99+71.05	16.75	726.46	
A	99+81.05	16.75	726.56	
B	99+91.05	16.75	726.61	
C	100+01.05	16.75	726.62	
D	100+11.05	16.75	726.57	
E	100+21.05	16.75	726.47	
CL Brg. N. Abut.	100+27.05	16.75	726.39	
N. End of Deck	100+27.89	16.75	726.38	
Back of N. Abut.	100+28.90	16.75	726.36	

*Information to be provided by PBFTG manufacturer, See Special Provisions

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	PLOT DATE =	DRAWN - CJH	REVISED -
		CHECKED -	REVISED -

**CITY OF WHEATON
DUPAGE COUNTY, ILLINOIS**

**TOP OF DECK ELEVATION TABLES
STRUCTURE NO. 022-7205**

SHEET 4 OF 20 SHEETS

M.S. RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	19

WEST EDGE OF GUTTER

Location	Station	Offset	Theoretical Grade Elevations
S. End South Appr. Pav't.	99+36.50	-13.50	725.77
A1	99+46.50	-13.50	726.05
A2	99+56.50	-13.50	726.28
N. End South Appr. Pav't.	99+66.50	-13.50	726.46

WEST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
S. End South Appr. Pav't.	99+36.68	-12.00	725.81
A1	99+46.68	-12.00	726.08
A2	99+56.68	-12.00	726.31
N. End South Appr. Pav't.	99+66.68	-12.00	726.49

℄ ROADWAY & PGL

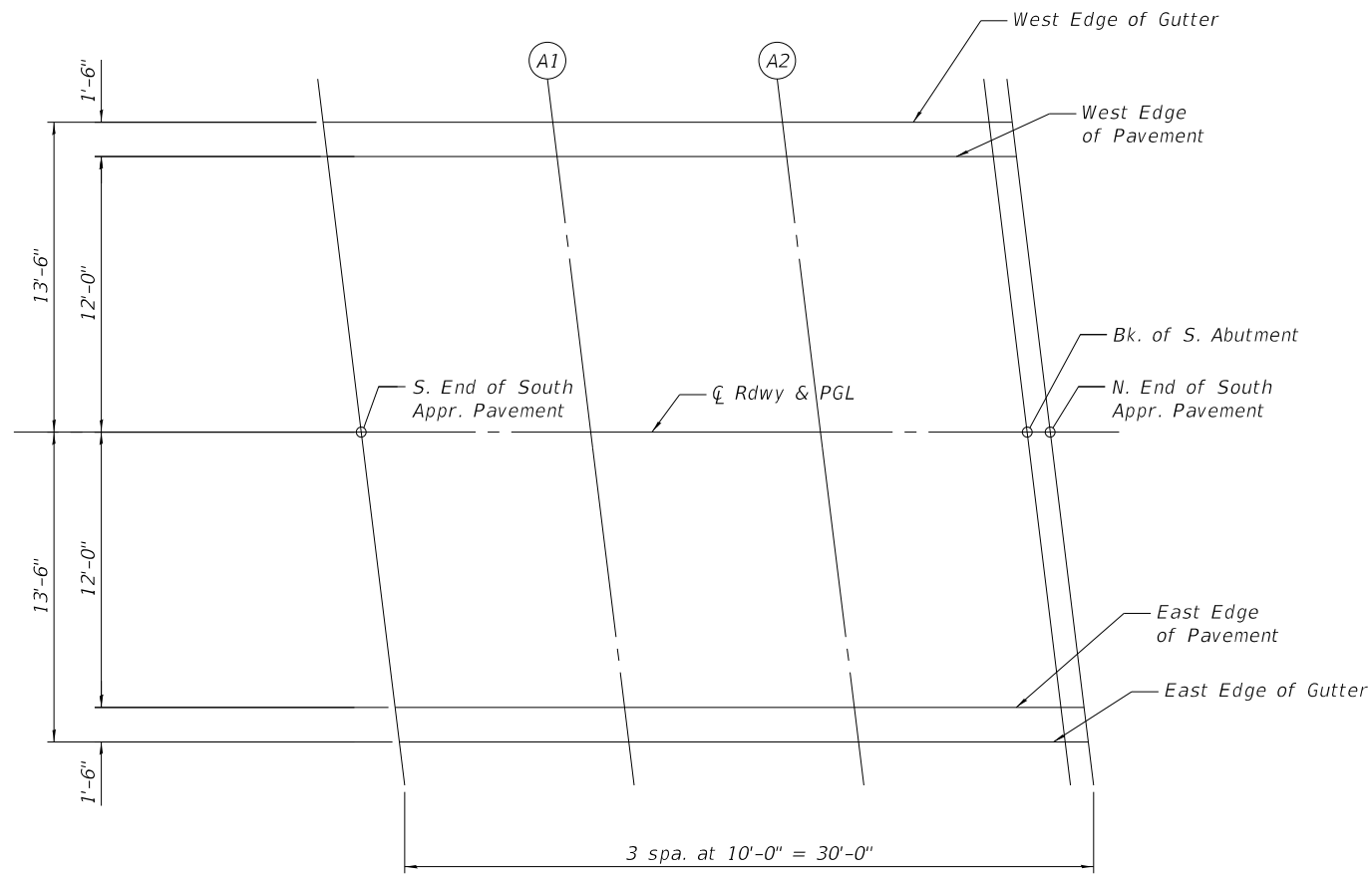
Location	Station	Offset	Theoretical Grade Elevations
S. End South Appr. Pav't.	99+38.16	0.00	726.09
A1	99+48.16	0.00	726.36
A2	99+58.16	0.00	726.58
N. End South Appr. Pav't.	99+68.16	0.00	726.75

EAST EDGE OF PAVEMENT

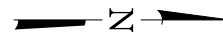
Location	Station	Offset	Theoretical Grade Elevations
S. End South Appr. Pav't.	99+39.63	12.00	725.89
A1	99+49.63	12.00	726.15
A2	99+59.63	12.00	726.37
N. End South Appr. Pav't.	99+69.63	12.00	726.53

EAST EDGE OF GUTTER

Location	Station	Offset	Theoretical Grade Elevations
S. End South Appr. Pav't.	99+39.82	13.50	725.87
A1	99+49.82	13.50	726.13
A2	99+59.82	13.50	726.34
N. End South Appr. Pav't.	99+69.82	13.50	726.50



PLAN
South Approach (NB)



Note:
For Bk. of Abutment layout see sheet 3 of 20.

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1170 SOUTH HOUBOLT ROAD
JOLIET, ILLINOIS 60431
(815) 744-4200
IDFPR NO. 184-001273

USER NAME =	DESIGNED - MJD	REVISED -
	CHECKED - AJS	REVISED -
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PLOT DATE =	CHECKED -	REVISED -

**CITY OF WHEATON
DUPAGE COUNTY, ILLINOIS**

**TOP OF SOUTH APPROACH SLAB ELEVATIONS
STRUCTURE NO. 022-7205**

M.S. RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	20

SHEET 5 OF 20 SHEETS

WEST EDGE OF GUTTER

Location	Station	Offset	Theoretical Grade Elevations
S. End North Appr. Pav't.	100+24.18	-13.50	726.50
A3	100+34.18	-13.50	726.33
A4	100+44.18	-13.50	726.12
N. End North Appr. Pav't.	100+54.18	-13.50	725.85

WEST EDGE OF PAVEMENT

Location	Station	Offset	Theoretical Grade Elevations
S. End North Appr. Pav't.	100+24.36	-12.00	726.53
A3	100+34.36	-12.00	726.36
A4	100+44.36	-12.00	726.14
N. End North Appr. Pav't.	100+54.36	-12.00	725.87

CL ROADWAY & PGL

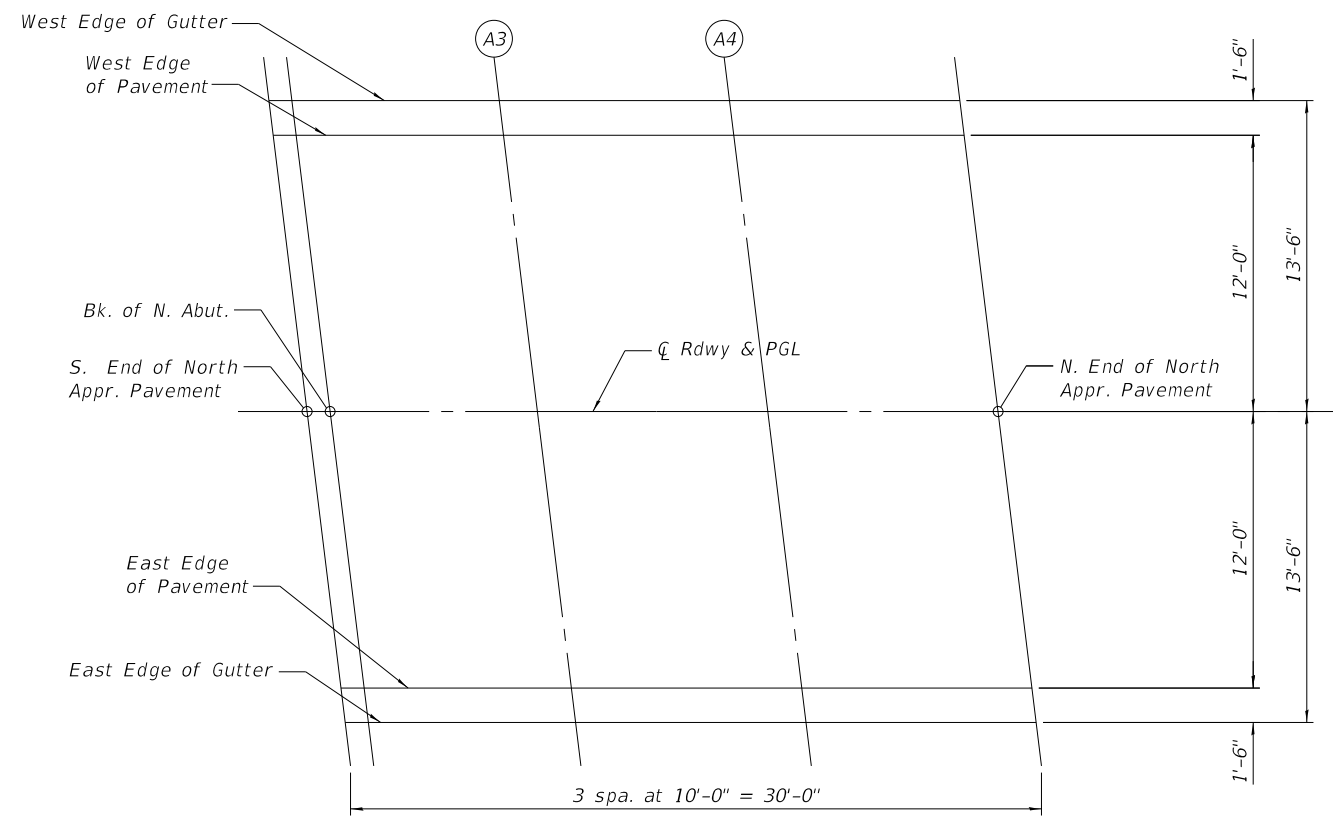
Location	Station	Offset	Theoretical Grade Elevations
S. End North Appr. Pav't.	100+25.84	0.00	726.74
A3	100+35.84	0.00	726.57
A4	100+45.84	0.00	726.35
N. End North Appr. Pav't.	100+55.84	0.00	726.08

EAST EDGE OF PAVEMENT

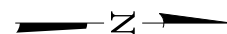
Location	Station	Offset	Theoretical Grade Elevations
S. End North Appr. Pav't.	100+27.31	12.00	726.48
A3	100+37.31	12.00	726.30
A4	100+47.31	12.00	726.07
N. End North Appr. Pav't.	100+57.31	12.00	725.80

EAST EDGE OF GUTTER

Location	Station	Offset	Theoretical Grade Elevations
S. End North Appr. Pav't.	100+27.49	13.50	726.45
A3	100+37.49	13.50	726.27
A4	100+47.49	13.50	726.04
N. End North Appr. Pav't.	100+57.49	13.50	725.76



PLAN



Note:
For Bk. of Abut. layout see sheet 3 of 20.

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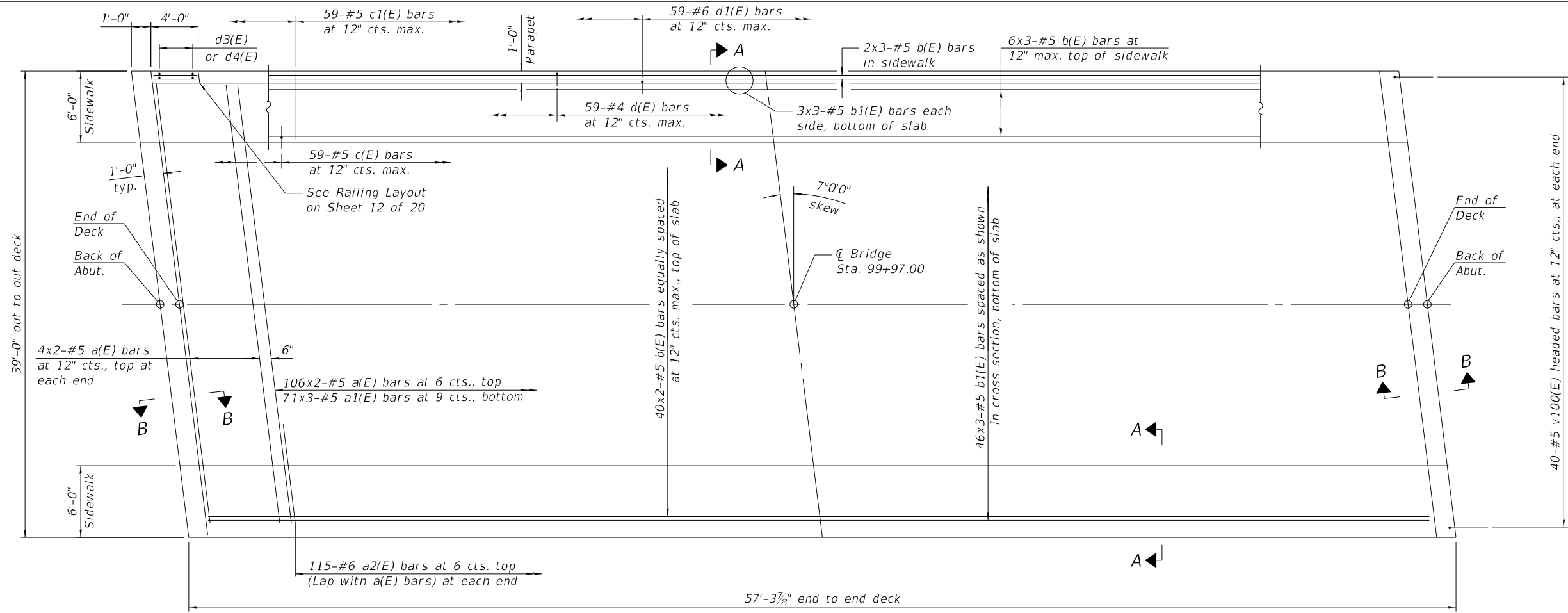
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	CHECKED -	REVISED -

**CITY OF WHEATON
DUPAGE COUNTY, ILLINOIS**

**TOP OF NORTH APPROACH SLAB ELEVATIONS
STRUCTURE NO. 022-7205**

SHEET 6 OF 20 SHEETS

M.S. RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	21

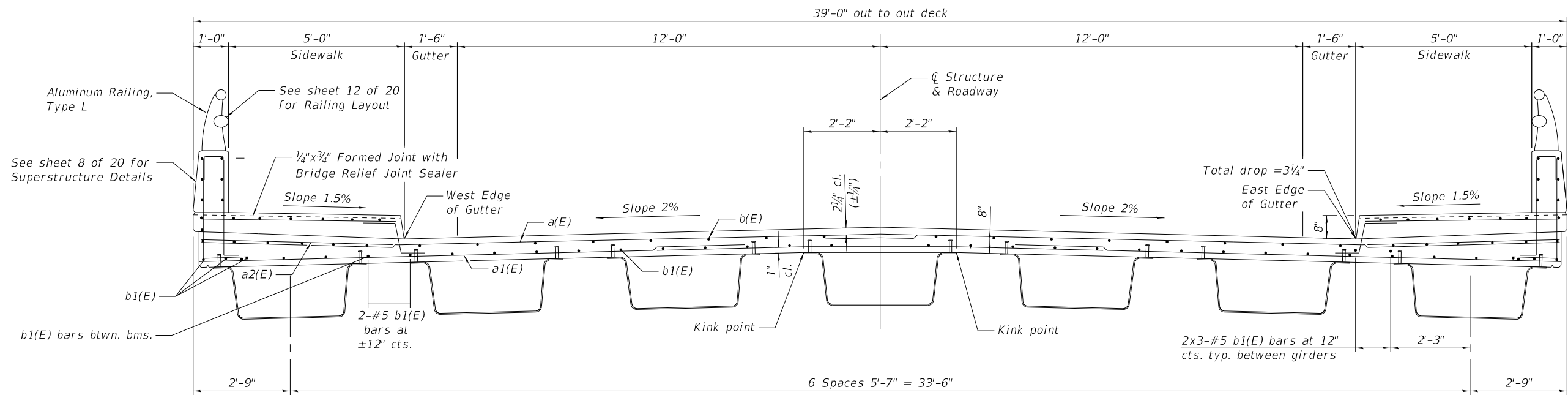


MINIMUM BAR LAP

- #4 bar = 2'-5"
- #5 bar = 3'-0"
- #6 bar = 3'-7"

PLAN

Notes:
 See sheet 8 of 20 for superstructure details and Bill of Material.
 Bars indicated thus 20x3-#5 etc. indicates 20 lines of bars with 3 lengths per line.



CROSS SECTION
(Looking north)

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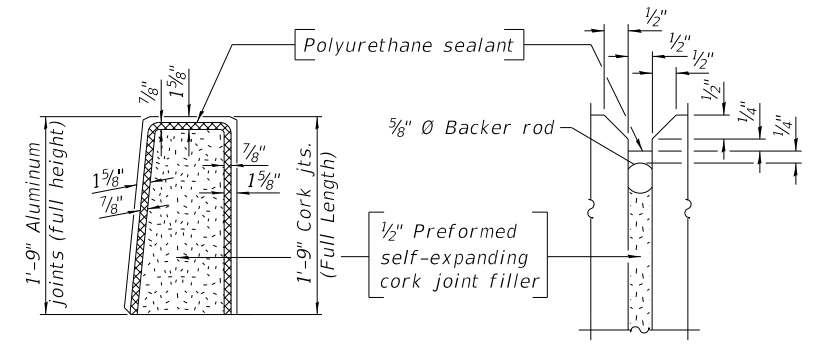
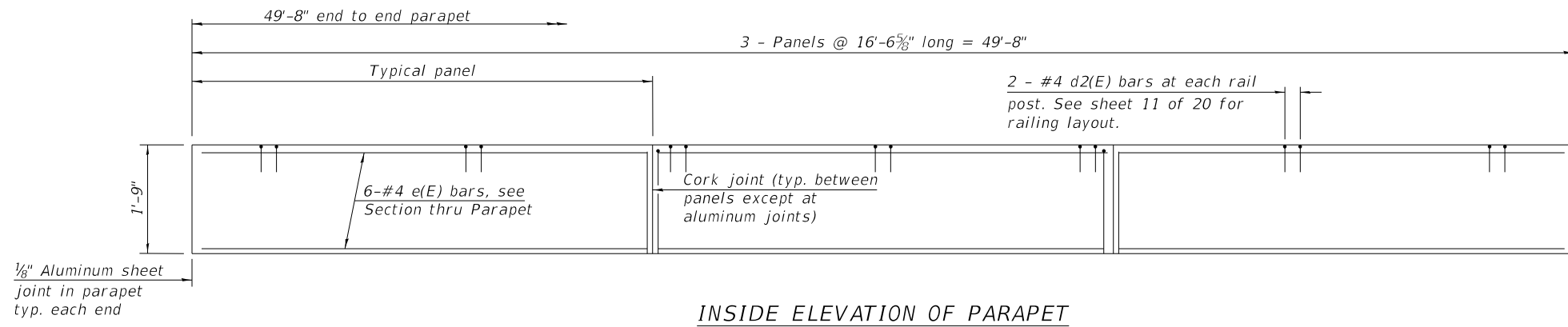
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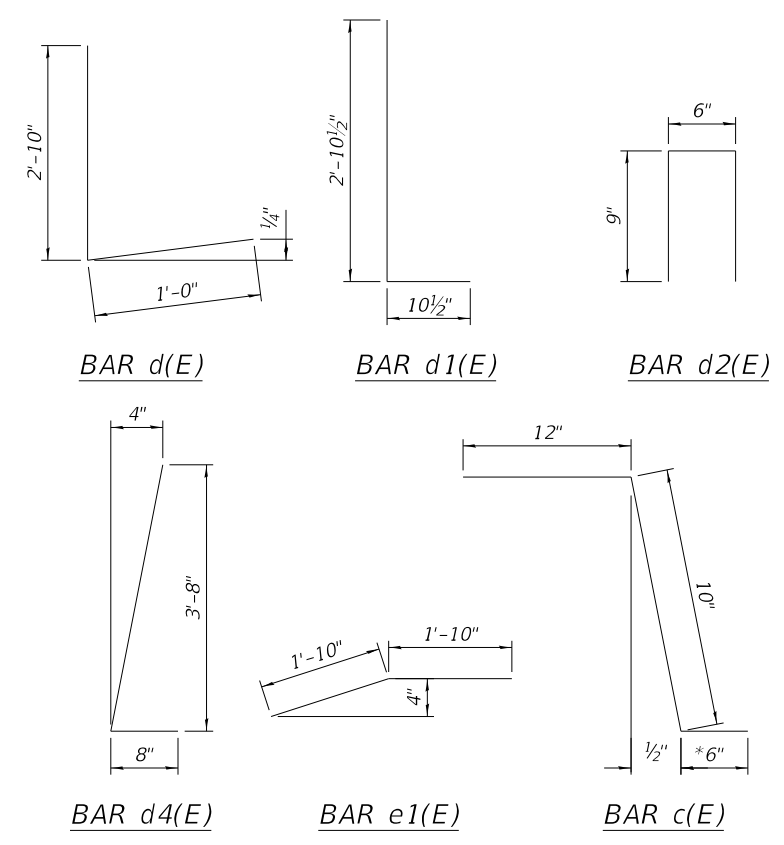
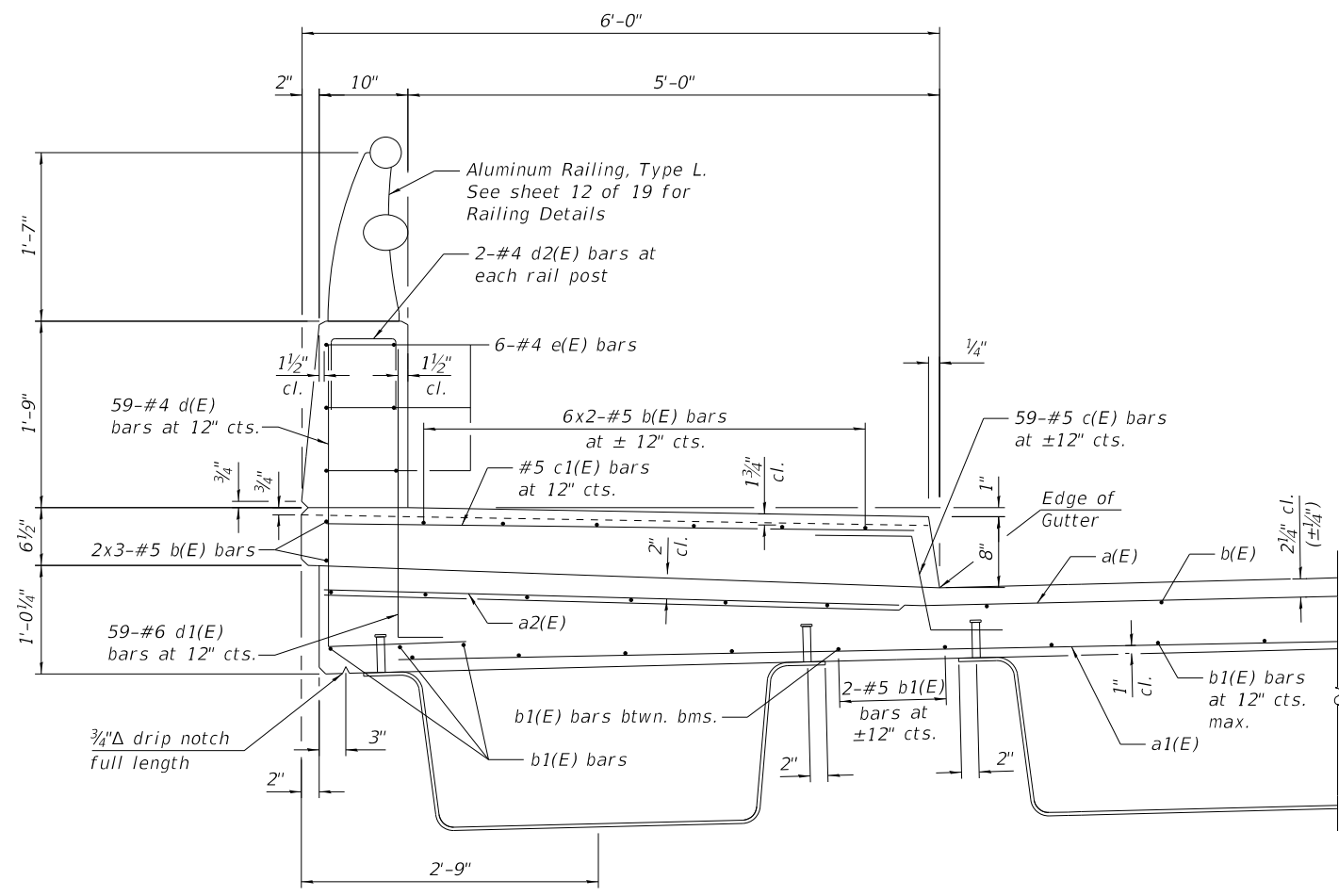
**SUPERSTRUCTURE
 STRUCTURE NO. 022-7205**

SHEET 7 OF 20 SHEETS

M.S. RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	22



MINIMUM BAR LAP
 #4 bar = 2'-5"



SUPERSTRUCTURE BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a(E)	228	#5	20'-10"	—
a1(E)	213	#5	14'-11"	—
a2(E)	232	#6	3'-7"	└
b(E)	104	#5	30'-0"	—
b1(E)	138	#5	21'-0"	—
c(E)	118	#5	2'-4"	└
c1(E)	118	#5	5'-8"	—
d(E)	118	#4	3'-10"	└
d1(E)	118	#6	3'-9"	└
d2(E)	28	#4	2'-0"	└
d3(E)	20	#4	3'-9"	L
d4(E)	20	#4	4'-4"	L
e(E)	36	#4	16'-3"	—
e1(E)	8	#4	3'-8"	└
e2(E)	32	#4	3'-8"	—
s10(E)	28	#6	5'-9"	└
s11(E)	28	#5	7'-6"	└
m10(E)	12	#6	21'-3"	—
m11(E)	28	#6	5'-4"	—
v100(E)	80	#5	3'-1"	└
Reinforcement Bars, Epoxy Coated			Pound	19,705
Bridge Deck Grooving			Sq. Yd.	160
Protective Coat			Sq. Yd.	277
Concrete Superstructure			Cy. Yd.	70.2

* In lieu of bottom leg, c(E) bars may be cored and set according to Article 509.06 of the Standard Specifications. Cored holes shall be roughed or scored per manufacturer's recommendations. Maximum depth of cored hole shall not exceed 6".

SECTION A-A

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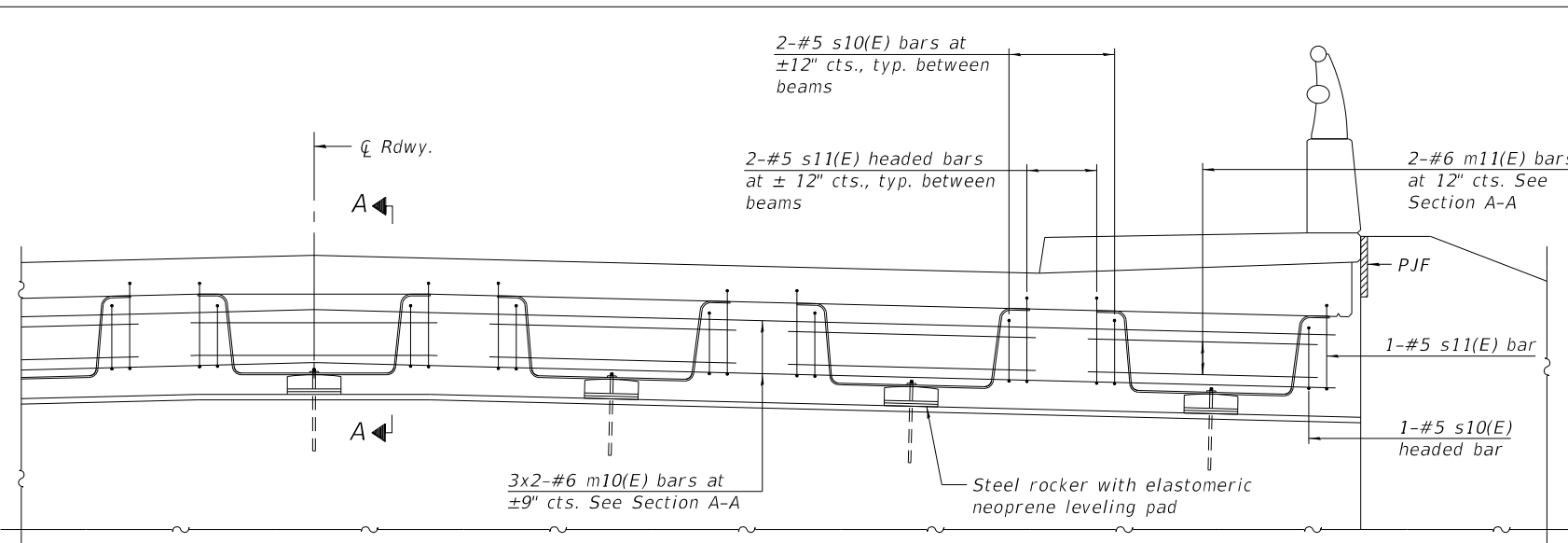
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**CITY OF WHEATON
 DUPAGE COUNTY, ILLINOIS**

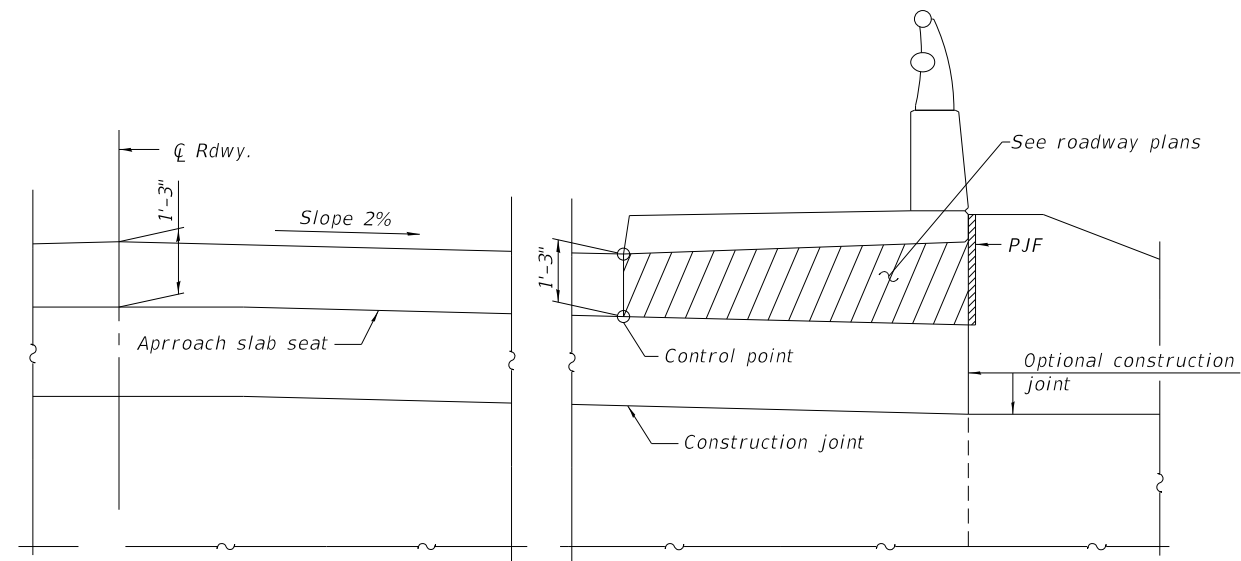
**SUPERSTRUCTURE DETAILS
 STRUCTURE NO. 022-7205**

SHEET 8 OF 20 SHEETS

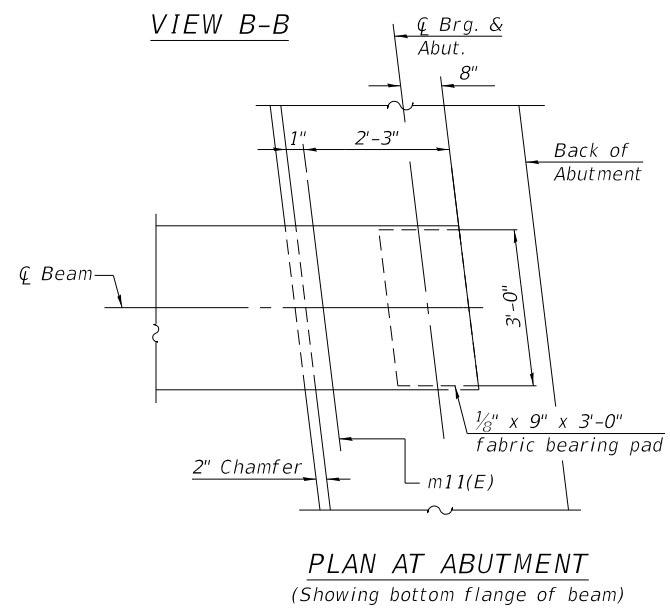
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4065	CREEKSIDE DRIVE	DUPAGE	47	23



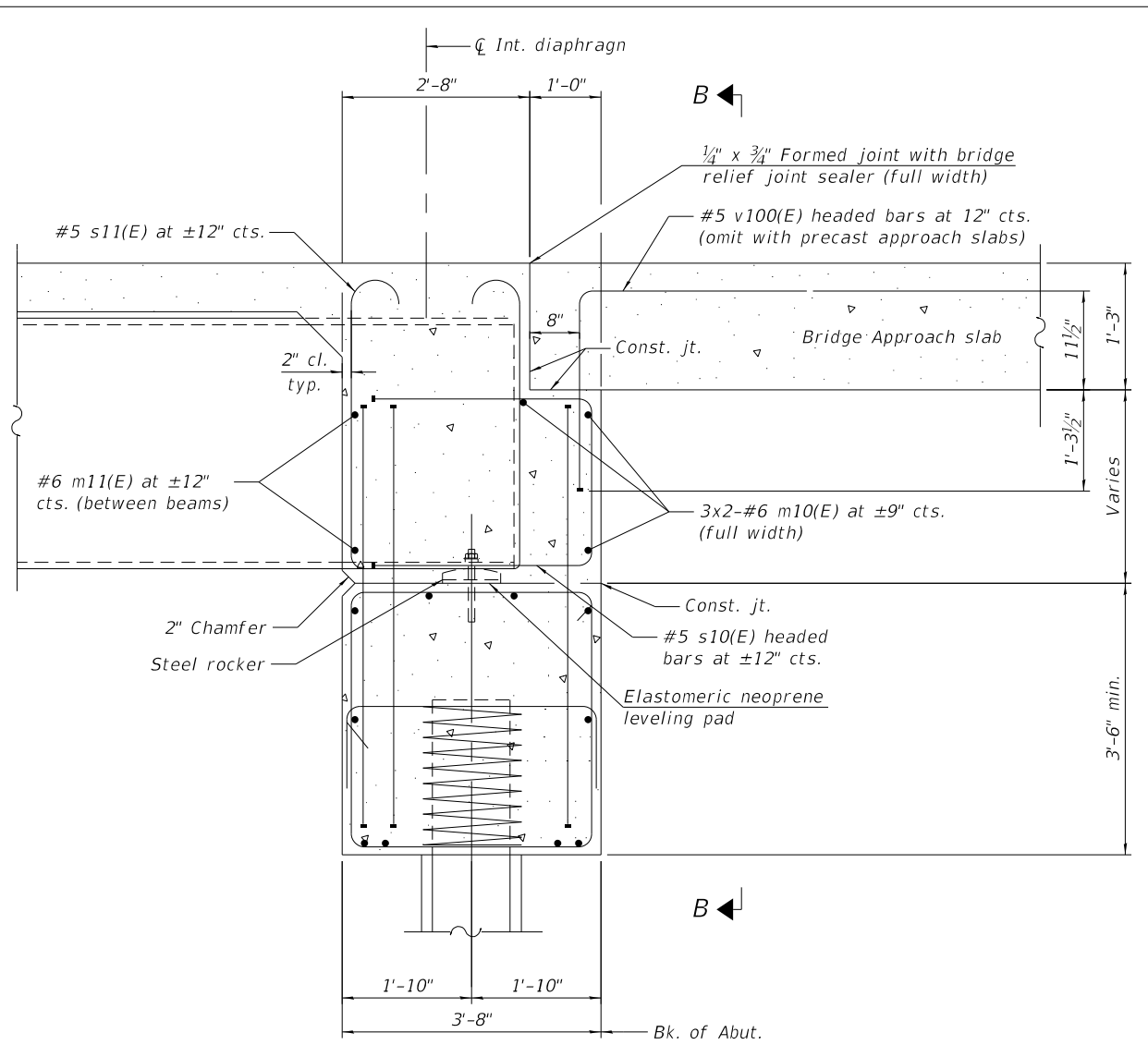
DIAPHRAGM AT ABUTMENT



VIEW B-B

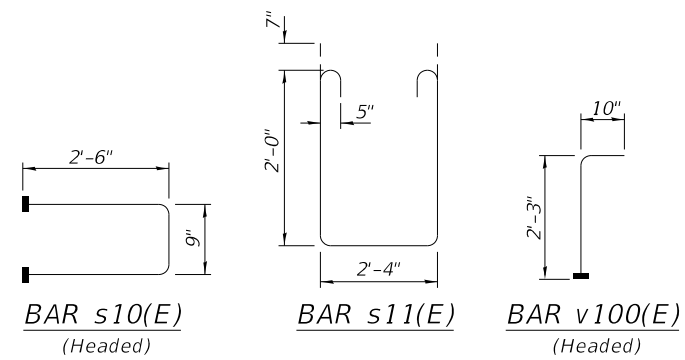


PLAN AT ABUTMENT
(Showing bottom flange of beam)



SECTION A-A

Note:
Headed bars shall conform to ASTM A970 with threaded attachment; Class HA; and reinforcement bars conforming to ASTM A706. Cost included with Reinforcement Bars, Epoxy Coated.
The s10(E), s11(E) and v100(E) bars are placed parallel to beams and spaced at right angles to beams.



Notes:
See sheet 8 of 20 for superstructure details and Bill of Material.
See sheet X of X for P.J.F. details.
The approach slab seat shall have a constant slope determined from the control points shown.
Cost of cellular polystyrene is included with Concrete Superstructure.

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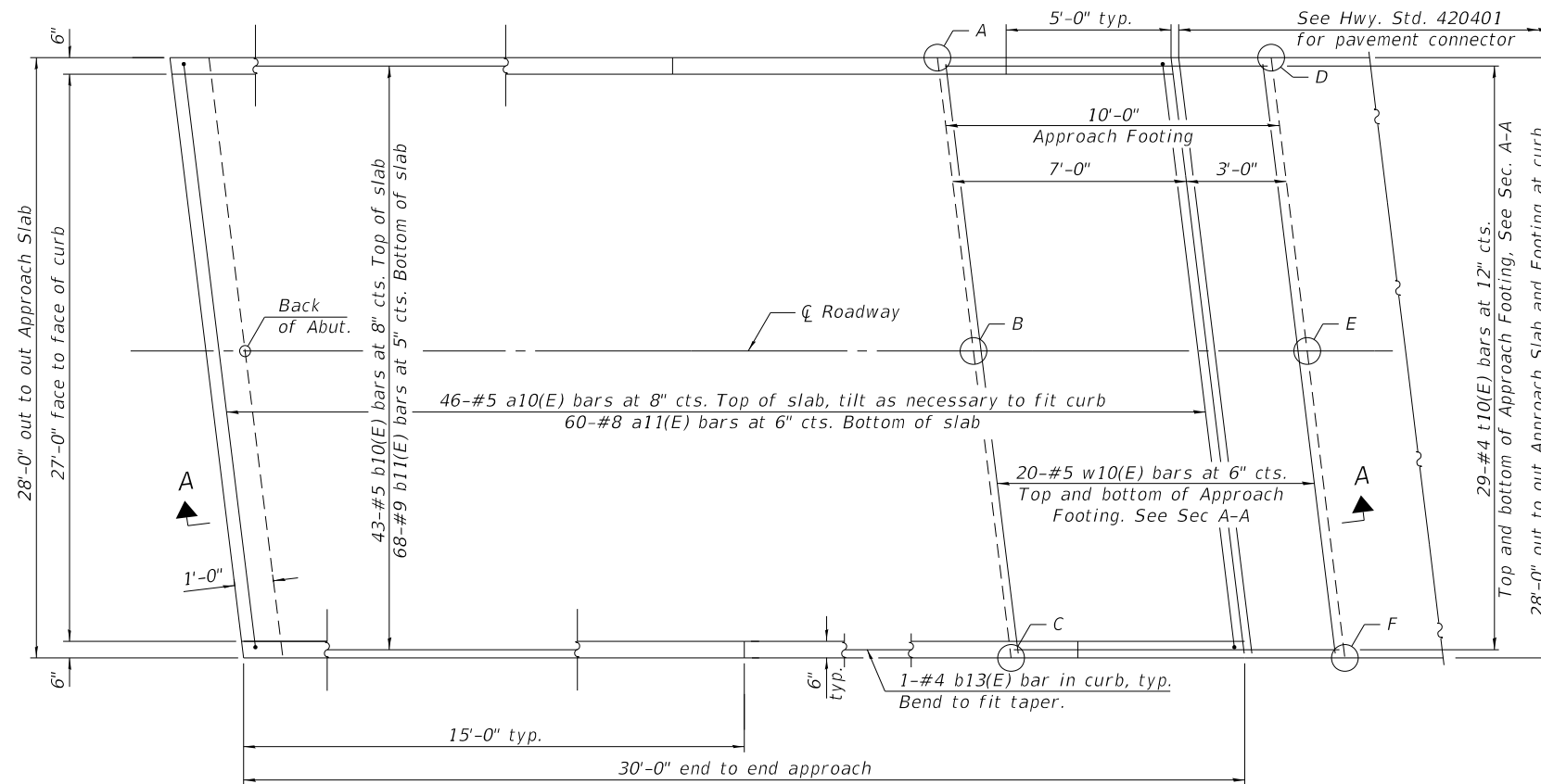
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		CHECKED -	REVISIONS

CITY OF WHEATON
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INTEGRAL ABUTMENT DIAPHRAGM DETAIL
STRUCTURE NO. 022-7205

SHEET 9 OF 20 SHEETS

M.S. RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	24



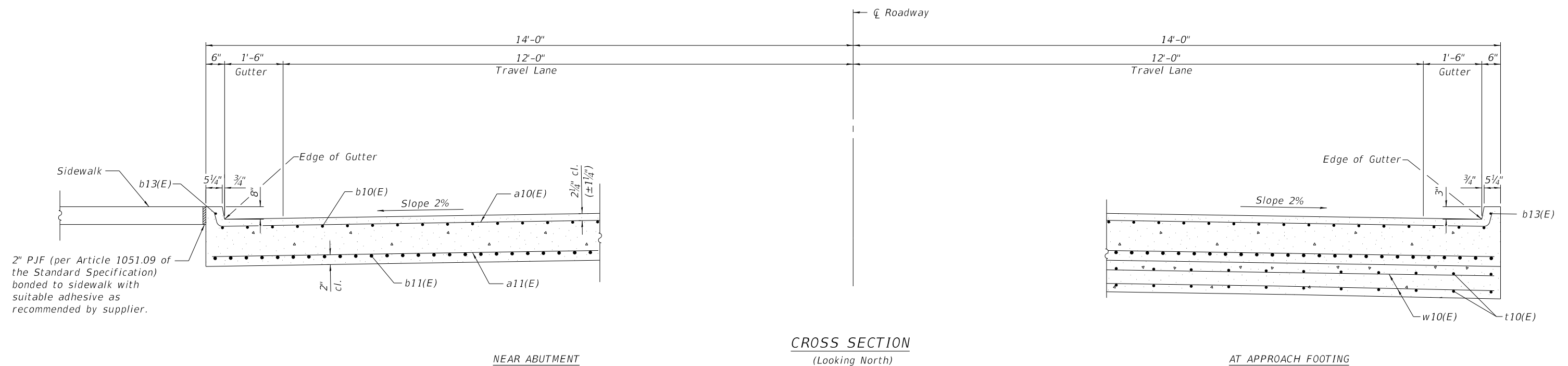
TOP AND BOTTOM ELEVATIONS
FOR APPROACH FOOTING

South Approach			North Approach		
Point	Top	Bottom	Point	Top	Bottom
A	724.81	723.98	A	724.78	723.94
B	725.04	724.21	B	725.01	724.18
C	724.71	723.88	C	724.69	723.85
D	724.52	723.69	D	724.51	723.68
E	724.75	723.92	E	724.75	723.92
F	724.43	723.59	F	724.42	723.59

PLAN

(North approach slab shown; South approach slab similar by 180° rotation)

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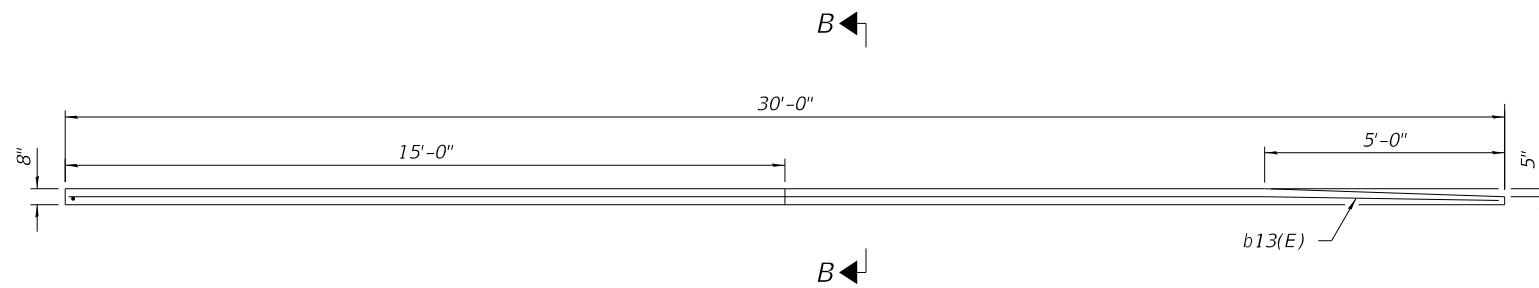
CROSS SECTION

(Looking North)

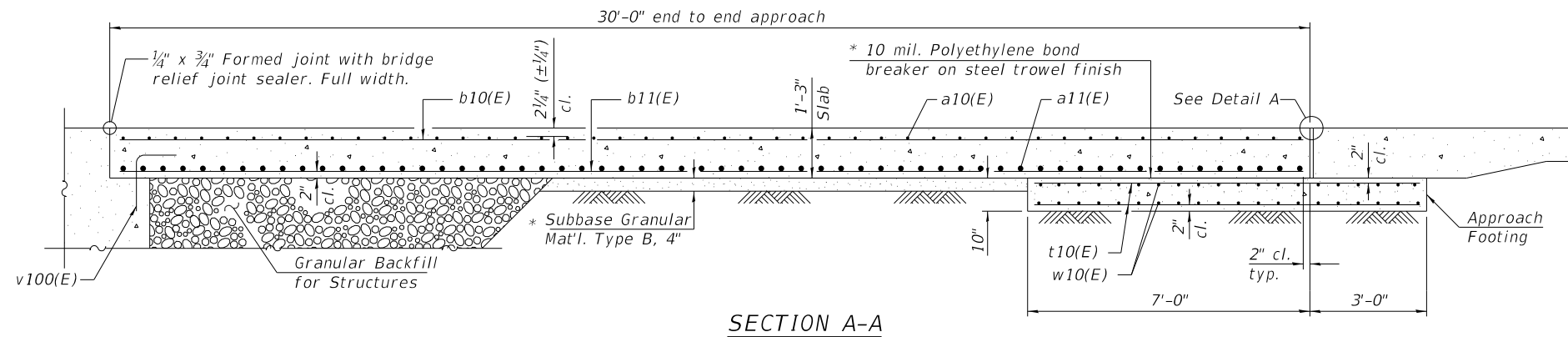
(Sheet 1 of 2)

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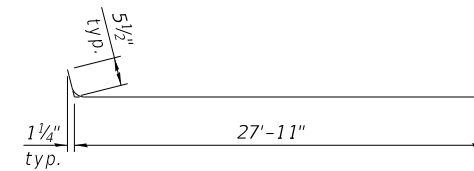
The joint opening shall be adjusted for temperature per Article 520.04 of the Standard Specifications. However, since this detail is for jointless structures, the length of bridge used to calculate the adjustment shall be equal to half the total bridge length plus the length of the bridge approach slab.
 Parapet concrete shall be paid for as Concrete Superstructure.
 Approach slab shall be paid for as Concrete Superstructure.
 Approach footing concrete shall be paid for as Concrete Structures.
 The approach footing maximum applied service bearing pressure (Q_{max}) = 2.0 ksf.
 Cost of excavation for approach footing included with Concrete Structures.
 For Granular Backfill for Structures and drainage treatment details, see sheet 2 of 20.



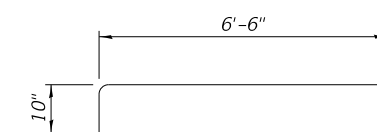
INSIDE ELEVATION OF CURB



SECTION A-A



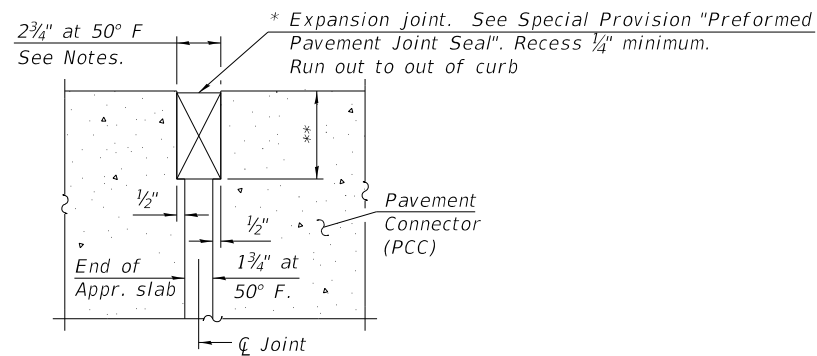
BAR a10(E)



BAR a12(E)

TWO APPROACHES
BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a10(E)	92	#5	28'-10"	┌───┐
a11(E)	120	#8	27'-11"	┌───┐
a12(E)	92	#5	7'-4"	┌───┐
b10(E)	86	#5	29'-8"	┌───┐
b11(E)	136	#9	29'-8"	┌───┐
b13(E)	8	#4	14'-8"	┌───┐
t10(E)	58	#4	9'-8"	┌───┐
w10(E)	80	#5	27'-11"	┌───┐
Concrete Structures			Cu. Yd.	17.3
Bridge Deck Grooving			Sq. Yd.	167
Protective Coat			Sq. Yd.	191
Concrete Superstructure (Approach Slab)			Cu. Yd.	77.8
Reinforcement Bars, Epoxy Coated			Pound	31,576



DETAIL A

* Cost included with Concrete Superstructure (Approach Slab).

** Per manufacturer recommendations

(Sheet 2 of 2)

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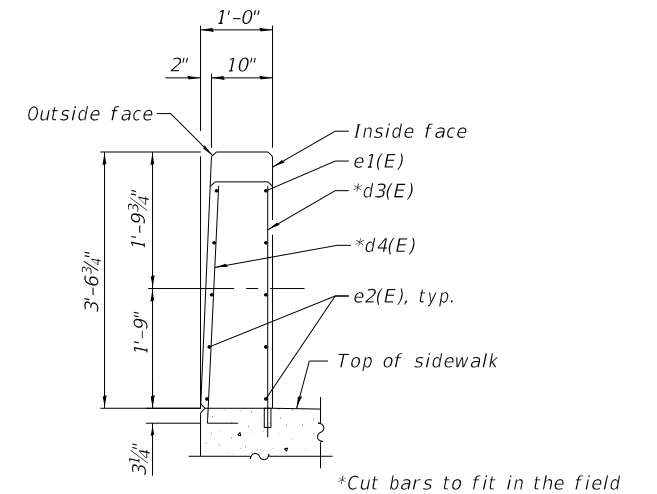
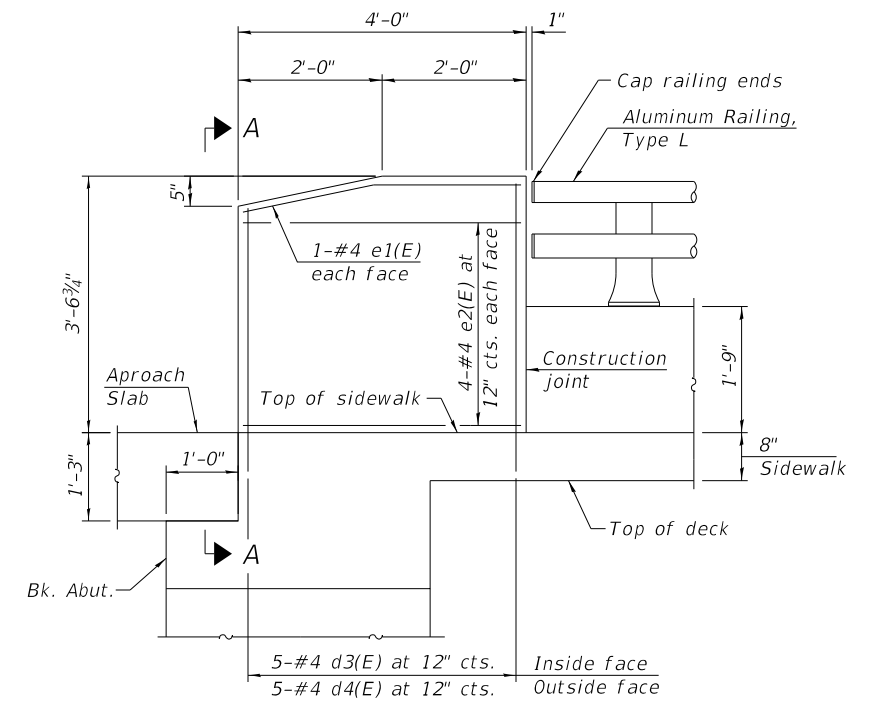
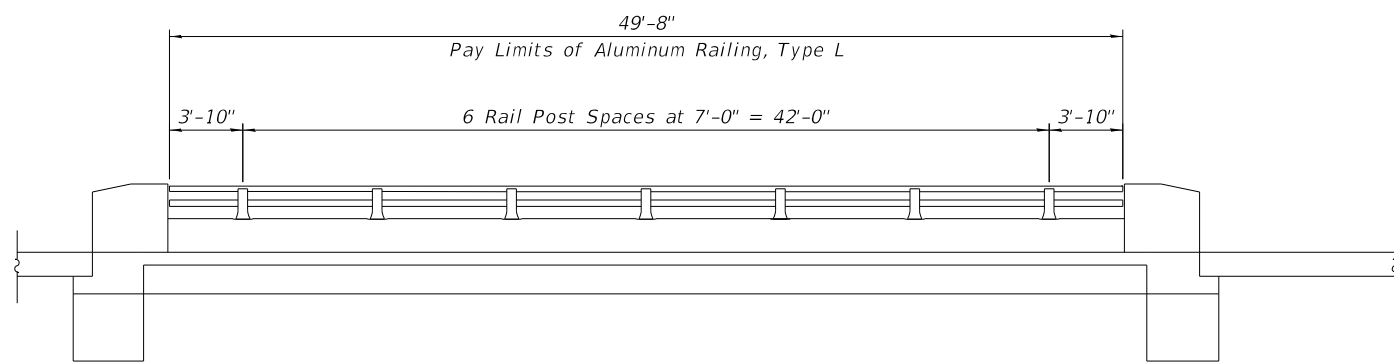
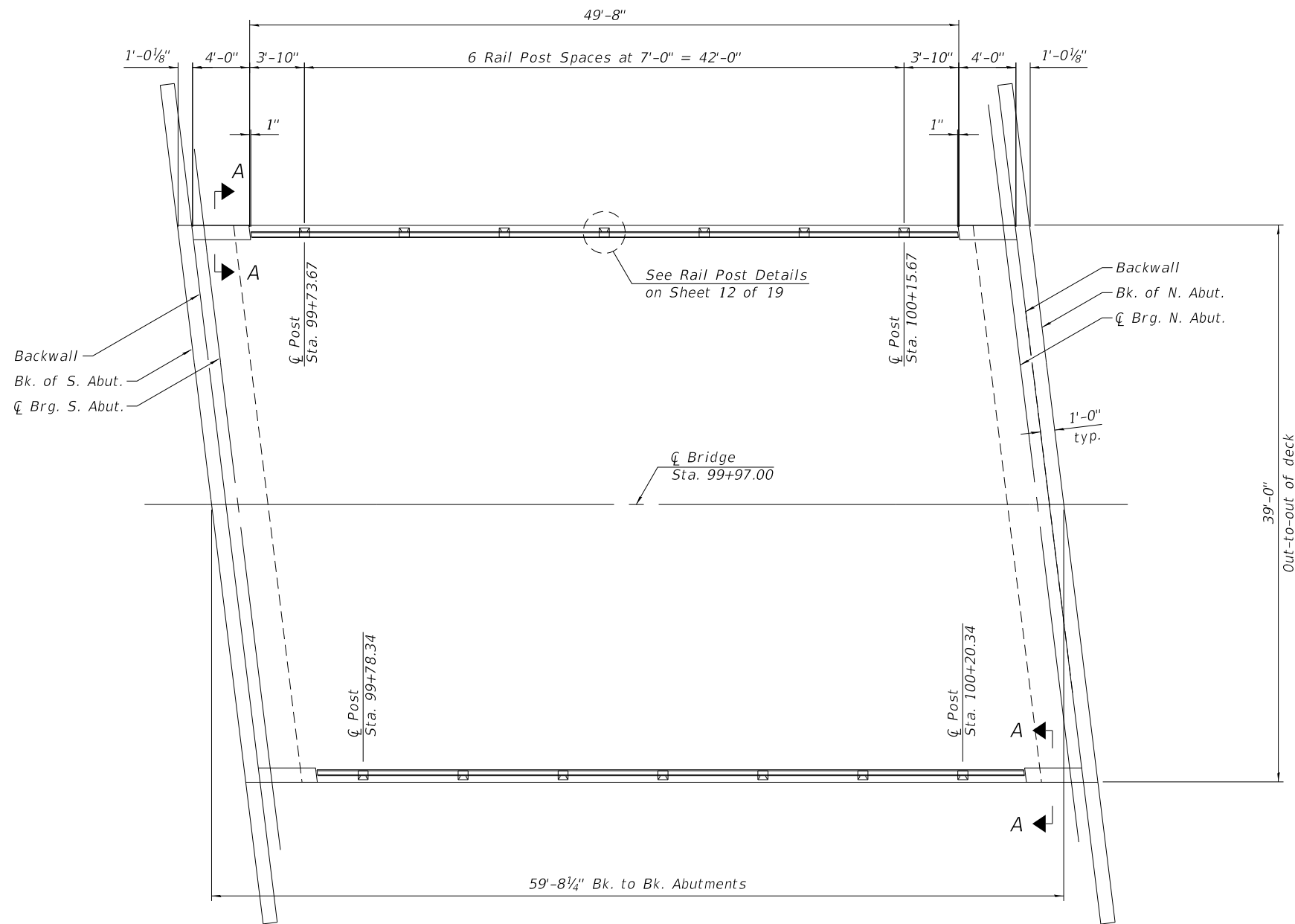
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BRIDGE APPROACH SLAB DETAILS (SHEET 2 OF 2)
STRUCTURE NO. 022-7205

SHEET 11 OF 20 SHEETS

M.S. RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	26



Note:
 Drill and set d3(E) bars in accordance with Article 509.06 of the Standard Specifications. Drilled holes shall be roughened or scored per manufacturers recommendation. Maximum depth shall not exceed 6".

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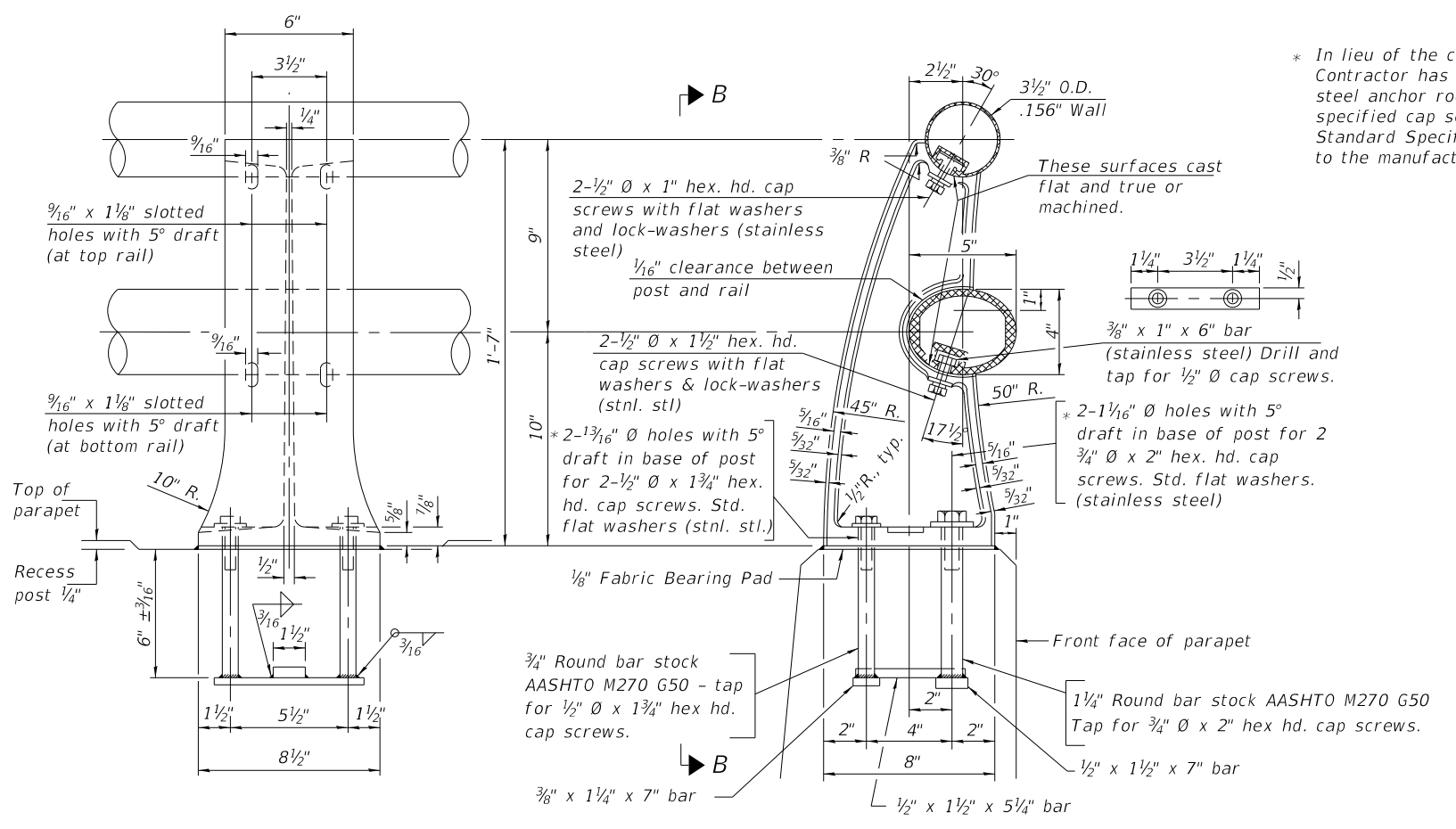
**CITY OF WHEATON
 DUPAGE COUNTY, ILLINOIS**

**RAILING LAYOUT (SHEET 1 OF 2)
 STRUCTURE NO. 022-7205**

SHEET 12 OF 20 SHEETS

M.S. RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	27

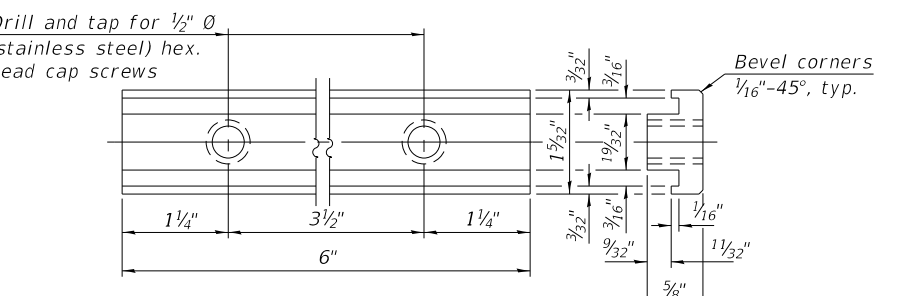
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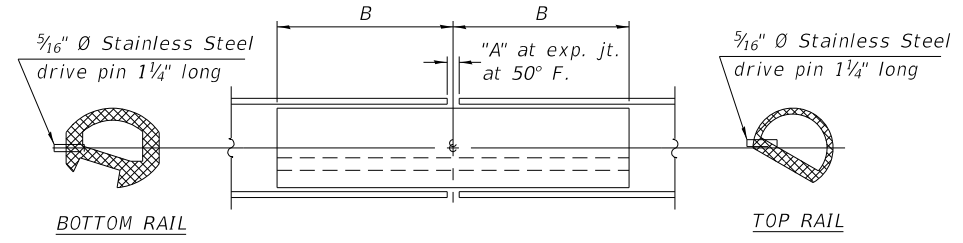
SECTION A-A

RAIL POST DETAILS



RAIL POST CLAMP BAR

For Top Rail

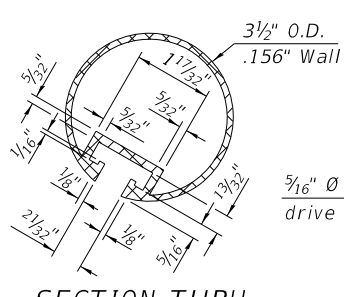


RAIL SPLICE

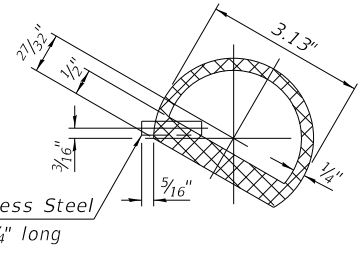
SPLICE DIMENSIONS

Location	T	A	B
All locs. not over exp. jts.	0	3/8"	1'-2"
Over Strip Seal Jt.	≤ 4"	2 1/2"	1'-2"
Over Finger or Modular Jt.	≤ 9 1/2"	5 1/2"	1'-7 3/4"
Over Finger or Modular Jt.	≤ 15"	8 1/4"	2'-1 1/4"

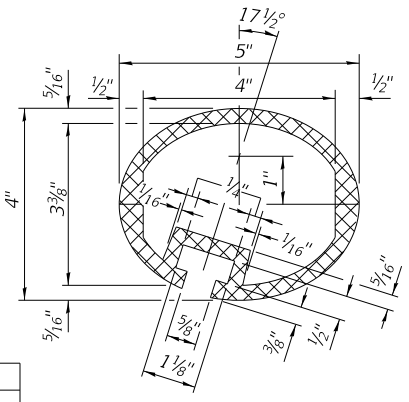
T = ; total movement along centerline of roadway at expansion joint.



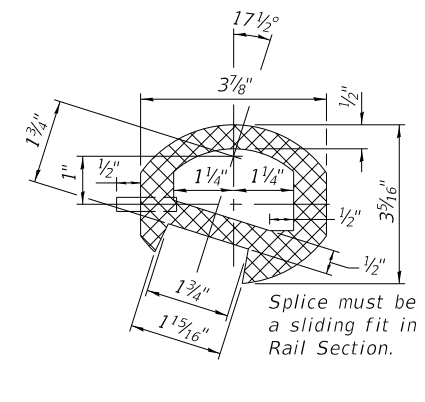
SECTION THRU TOP RAIL



SECTION THRU TOP RAIL SPLICE

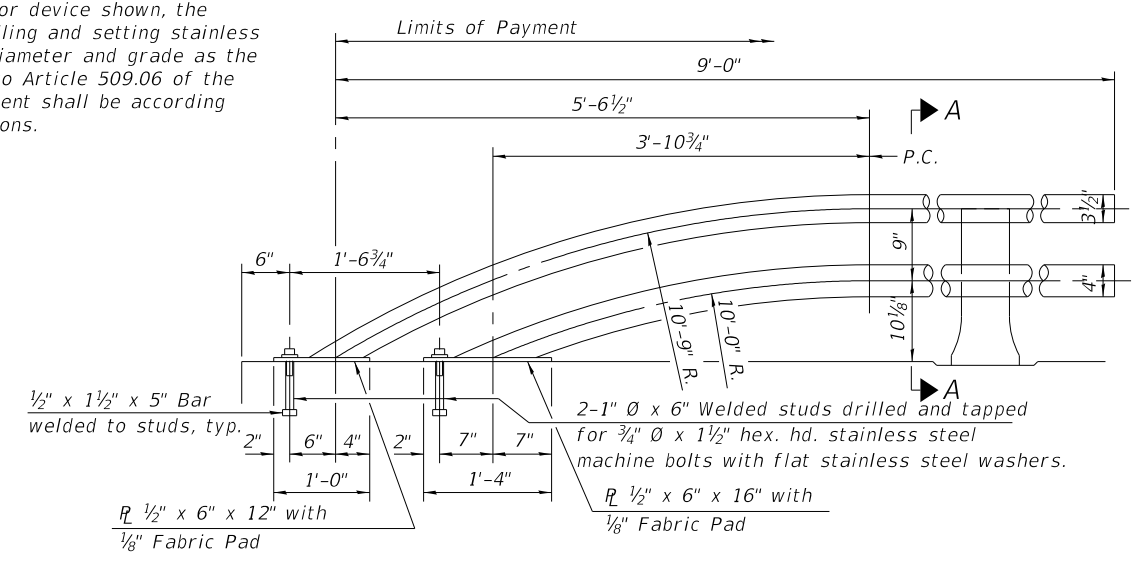


SECTION THRU BOTTOM RAIL



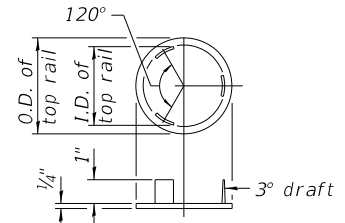
SECTION THRU BOTTOM RAIL SPLICE

* In lieu of the cast-in-place anchor device shown, the Contractor has the option of drilling and setting stainless steel anchor rods of the same diameter and grade as the specified cap screws according to Article 509.06 of the Standard Specifications. Embedment shall be according to the manufacturer's specifications.

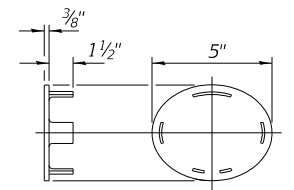


RAIL TERMINAL SECTION

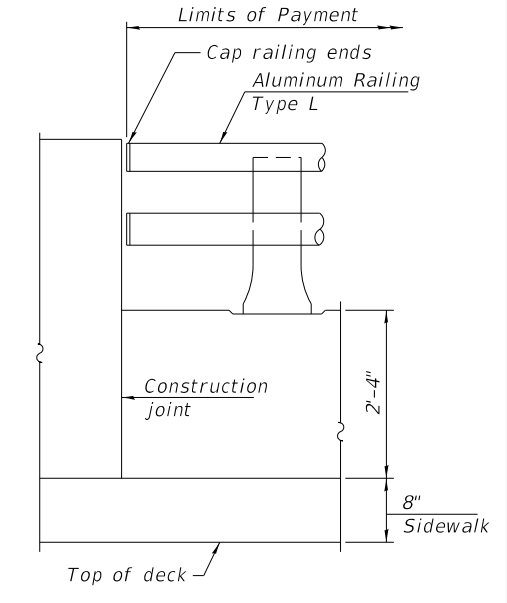
Note: The end rail post shall be set back as required for the terminal rail section.



CAST END CAP For top rail Drive Fit Type



CAST END CAP For bottom rail Drive Fit Type



RAIL END TREATMENT FOR TYPE 5 AND 6 TERMINAL

BILL OF MATERIAL

Item	Unit	Quantity
Aluminum Railing, Type L	Foot	100

Notes:
 All Posts shall be normal to parapet.
 All joints in rail shall be spliced per detail.
 All exposed rail ends shall be capped per detail.
 Provide 1-1/8" and 2-1/16" Aluminum Shims for 25% of the Posts. Rail elements shall be parallel to Grade, high spots shall be ground and low spots shimmed.
 Place reinforcement bars to miss anchor rod locations.
 See sheet 12 of 20 for rail post spacing.

RAILING CRITERIA

NCHRP 350 Test Level	4
Post Spacing Range	7'-0" - 10'-0"
Rail Weight (plf)	40

R-20 10-12-2021

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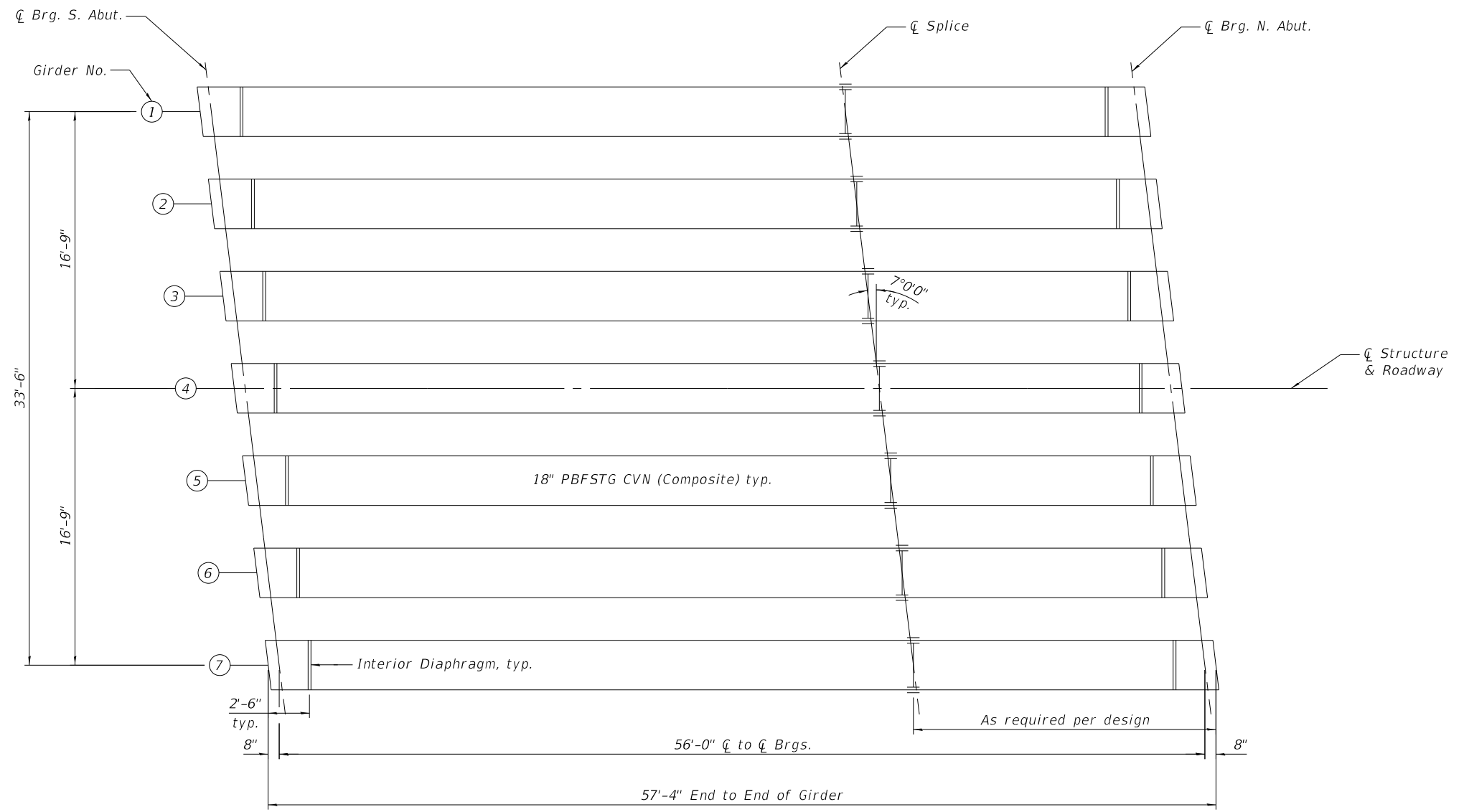
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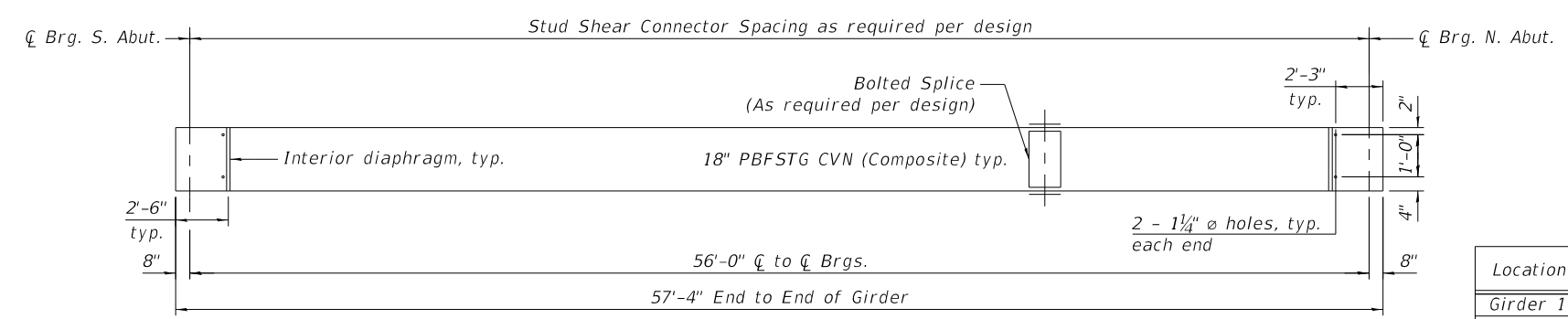
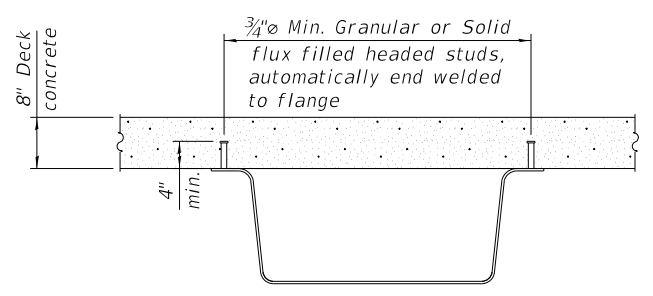
**RAILING DETAILS (SHEET 2 OF 2)
 STRUCTURE NO. 022-7205**

SHEET 13 OF 20 SHEETS

M.S. RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSID DRIVE	DUPAGE	47	28



FRAMING PLAN



GIRDER ELEVATION

Notes:
 All primary members shall be M270 Grade 50. All secondary members shall be M270 Grade 50.
 For additional structural steel details see sheet 15 of 20.
 All girders shall be braced for stability during girder installation as required per Manufacturer.
 "CVN" denotes Charpy-V-Notch impact energy requirements, Zone 2.
 All Structural Steel and H.S. Bolts shall be galvanized according to the Special Provisions.
 Bolted splice shall not be permitted within the middle third of the span length.

Location	℄ Brg. S. Abut.	℄ Brg. N. Abut.
Girder 1		
Girder 2		
Girder 3		
Girder 4		
Girder 5		
Girder 6		
Girder 7		

TOP OF WEB ELEVATIONS
 (For fabrication only)
 (Does not include Dead Load Deflection)
 Top of Web Elevations to be determined/filled out by PBFSTG Designer/Manufacturer if required.

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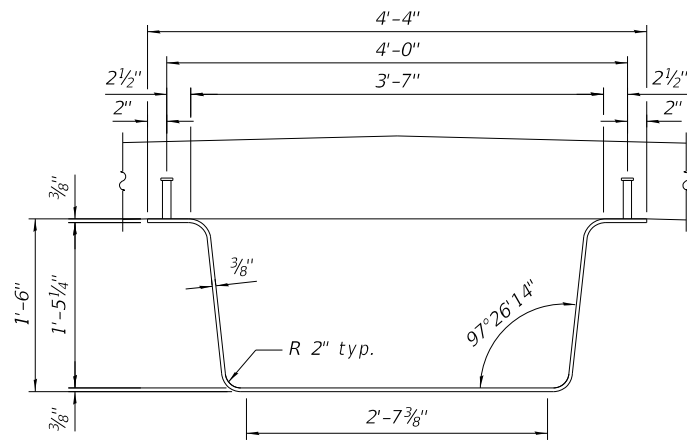
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	CHECKED - AJS	REVISED -
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**CITY OF WHEATON
 DUPAGE COUNTY, ILLINOIS**

**STRUCTURAL STEEL
 STRUCTURE NO. 022-7205**

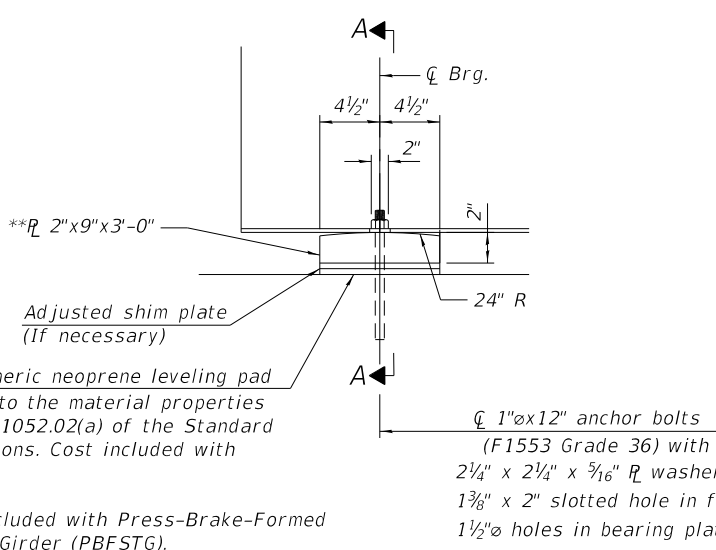
SHEET 14 OF 20 SHEETS

M.S. RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	29

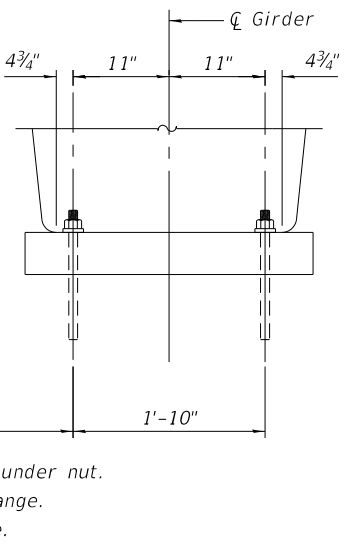


ESTIMATED STEEL SECTION

Notes:
 Two 1/8" adjusting shims shall be provided for each bearing in addition to all other plates or shims and places as shown on bearing details.
 Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.
 Anchor bolts at fixed bearings may be either cast in place or installed in holes drilled after the supported member is in place
 Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.
 The structural steel plates of the fixed bearings, including pintles (if applicable), shall conform to the requirements of AASHTO M270 Grade 50.
 Anchor bolts at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.
 "CVN" denotes Charpy-V-Notch impact energy requirements, Zone 2.
 All primary members shall be A572 Grade 50. All secondary members shall be M270 Grade 50.
 All structural steel and H.S. bolts shall be galvanized according to the Special Provisions.
 Abutment design and details is based on assumed typical reactions and dimensions. Contractor shall verify that final design and details are compatible with the selected superstructure prior to construction. The Contractor shall employ a Structural Engineer licensed in the state of Illinois to provide alternate abutment designs as required at no additional cost to the contract.
 All bearing plates, anchor bolts, nuts, washers and pintles shall be galvanized according to AASHTO M111 or M232 as applicable.
 Bearing plate width is based on plate layout along the C of bearing. Abutment width allows for bearing plate aligned perpendicular to girder. Adjustment is allowed if needed for design of PBFSTG.



ELEVATION AT ABUT.



SECTION A-A

(Horiz. dimensions at Rt. L's to C Girder)

FIXED BEARINGS AT ABUTMENTS

(14 Required)

*INTERIOR GIRDER MOMENT TABLE	
	0.5 Sp. 1
Is	(in ⁴)
Ic(n)	(in ⁴)
Ic(3n)	(in ⁴)
Ss	(in ³)
Sc(n)	(in ³)
Sc(3n)	(in ³)
DC1	(k')
MDC1	(k)
DC2	(k')
MDC2	(k)
DW	(k')
MDW	(k)
LLDF	
M _{L+IM}	(k)
M _u (Strength I)	(k)
Øf Mn	(k)
f _s DC1	(ksi)
f _s DC2	(ksi)
f _s DW	(ksi)
f _s (L+IM)	(ksi)
f _s (Service II)	(ksi)
0.95Rh Fyf	(ksi)
f _s (Total)(Strength I)	(ksi)
Øf Fn	(ksi)
Vf	(k)

*INTERIOR GIRDER REACTION TABLE	
ABUTMENT	Interior
LLDF	
OCF	
RDC1 (kip)	
RDC2 (kip)	
RDW (kip)	
R LL (kip)	
R IM (kip)	
R LL (PEDESTRIAN) (kip)	
R TOTAL (kip)	

*Information to be provided by PBFSTG manufacturer. See Special Provisions.

ESTIMATED GIRDER REACTION TABLE	
ABUTMENT	Interior / Exterior
RDC1 (kip)	22
RDC2 (kip)	1
RDW (kip)	9
R LL (kip)	23
R IM (kip)	8
R LL (PEDESTRIAN) (kip)	3
R TOTAL (kip)	66

Non-composite moment of inertia and section modulus of the Is, Ss: steel section used for computing fs(Total-Strength I, and Service II) due to non-composite dead loads (in.4 and in.3).
 Composite moment of inertia and section modulus of the steel Ic(n), Sc(n): and deck based the modular ratio, "n", used for computing fs(Total-Strength I, and Service II) in uncracked sections due to short-term composite live loads (in.4 and in.3).
 Composite moment of inertia and sections modulus of the steel Ic(3n), Sc(3n): and deck based upon 3 times the modular ratio, "3n", used for computing fs(Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in.4 and in.3)
 Un-factored non-composite dead load (kips/ft.).
 DC1: Un-factored moment due to non-composite dead load (kip-ft.).
 MDC1: Un-factored long-term composite (superimposed excluding future DC2: wearing surface) dead load (kips/ft.).
 Unfactored moment due to long-term composite (superimposed MDC2: excluding future wearing surface) dead load(kip-ft.).
 Un-factored long-term composite (superimposed future wearing DW: surface only) dead load (kips/ft.).
 Un-factored moment due to long-term composite (superimposed MDW: future wearing surface only) dead load (kip-ft.).
 Un-factored live load moment plus dynamic load allowance (impact) MLL+1M: (kip-ft.).
 Factored design moment (kip-ft.).
 Mu (Strength I): 1.25 (MDC1 + MDC2) + 1.5 MDW + 1.75 M
 Compact composite positive moment capacity computed according of Mn: to Article 6.10.7.1 or non-slender negative moment capacity according to Article A6.1.1 or A6.1.2 (kip-ft.).
 Un-factored stress at edge of flange for controlling steel fs DC1: flange due to vertical non-composite dead loads as calculated below (ksi).
 MDC1/Snc
 Un-factored stress at edge of flange for controlling steel fs DC2: flange due to vertical composite dead loads as calculated below (ksi).
 MDC2/Sc(3n) or MDC2/Sc(cr) as applicable
 Un-factored stress at edge of flange for controlling steel fs DW: flange due to vertical composite future wearing surface loads as calculated below (ksi).
 MDW/Sc(3n) or MDW/Sc(cr) as applicable.
 Un-factored stress at edge of flange for controlling steel fs (LL+1M): flange due to vertical composite live load plus impact loads as calculated below (ksi).
 M /Sc(n) or M /Sc(cr) as applicable
 Sum of stresses as computed below (ksi).
 fs (Service II): fsDC1 +fsDC2 + fsDW + 1.3 fs()
 Composite stress capacity for Service II loading according 0.95rHfYF: to Article 6.10.4.2 (ksi).
 Sum of stresses as computed below on non-compact fs (Total)(Strength I): section (ksi).
 1.25 (fsDC1 + fsDC2) + 1.5 Fsdw + 1.75 fs()
 Non-Compact composite positive or negative stress capacity for of Fn: Strength I loading according to Article 6.10.7 or 6.10.8 (ksi).
 Maximum factored shear range in span computed according VF: to Article 6.10.10
 LLDF: Live load Distribution Factor
 OCF: Obtuse correction Factor

BILL OF MATERIAL

Item	Unit	Quantity
Press-Brake-Formed Steel Tub Girder (PBFSTG)	Pound	41,619
Anchor Bolts, 1"	Each	28

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1170 SOUTH HOUBOLT ROAD
 JOLIET, ILLINOIS 60431
 (815) 744-4200
 IDFPNR NO. 184-001273

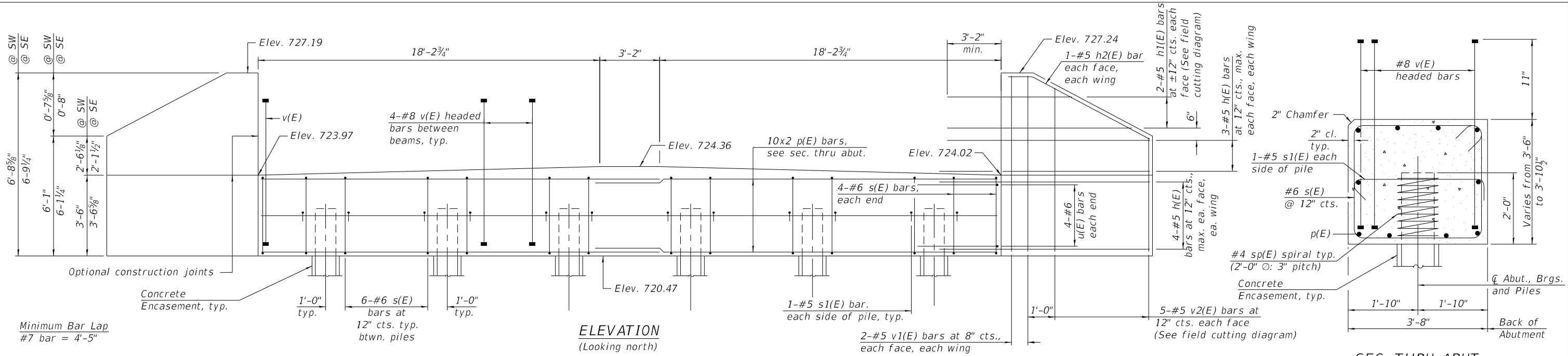
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CITY OF WHEATON
 DUPAGE COUNTY, ILLINOIS

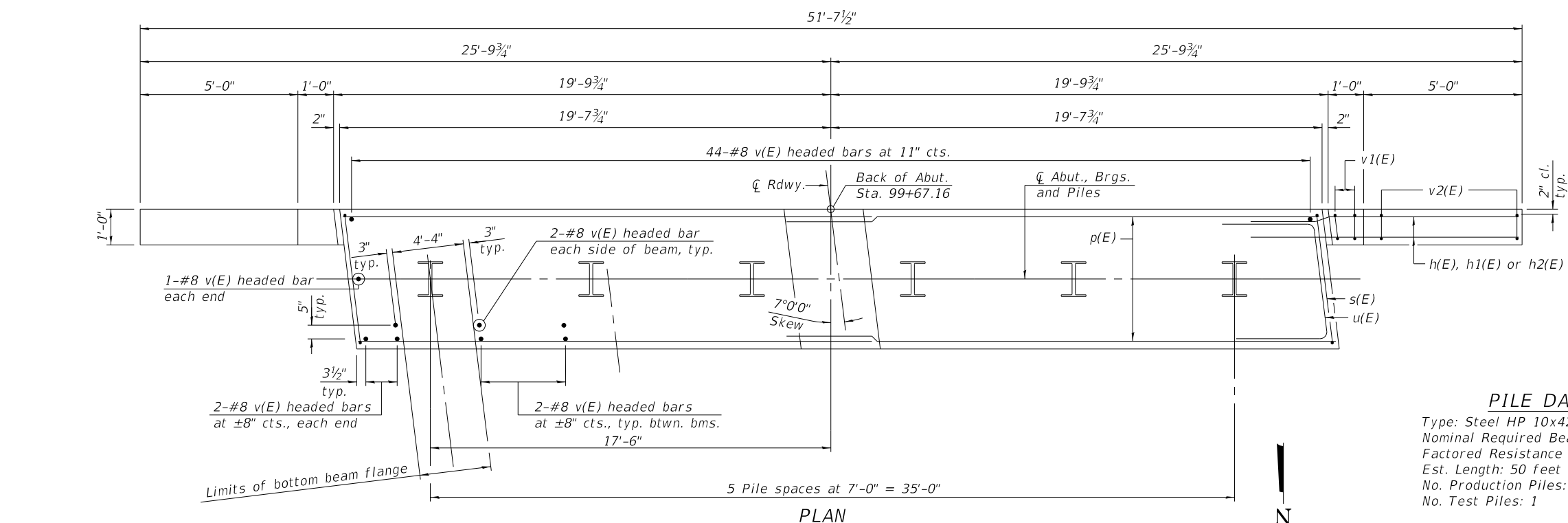
STRUCTURAL STEEL DETAILS
 STRUCTURE NO. 022-7205

SHEET 15 OF 20 SHEETS

M.S. RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	30



SEC. THRU ABUT.



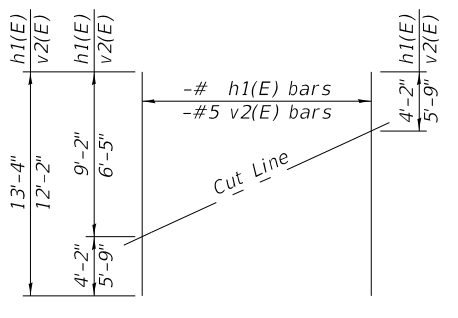
PLAN

BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h(E)	12	#5	9'-2"	—
h1(E)	4	#5	13'-4"	—
h2(E)	4	#5	5'-8"	—
p(E)	20	#7	21'-8"	—
s(E)	38	#6	14'-4"	□
s1(E)	12	#5	4'-4"	└┘
u(E)	8	#6	11'-10"	□
v(E)	74	#8	4'-3"	—
v1(E)	8	#5	6'-5"	—
v2(E)	10	#5	12'-2"	—
sp(E)	6	#4	2'-0"	—
Structure Excavation			Cu. Yd.	67
Concrete Structures			Cu. Yd.	22.6
Concrete Encasement			Cu. Yd.	2
Reinforcement Bars, Epoxy Coated			Pound	3122
Furnishing Steel Piles HP10x42			Foot	260
Driving Piles			Foot	250
Test Pile Steel HP10x42			Each	1
Pile Shoes			Each	6
Granular Backfill for Structures			Cu. Yd.	14
Geocomposite Wall Drain			Sq. Yd.	27
Pipe Underdrain for Structures 4"			Foot	51

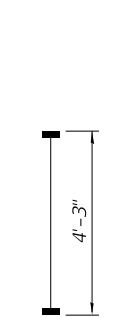
PILE DATA

Type: Steel HP 10x42 with Pile Shoes
 Nominal Required Bearing: 332 kips
 Factored Resistance Available:
 Est. Length: 50 feet
 No. Production Piles: 5
 No. Test Piles: 1

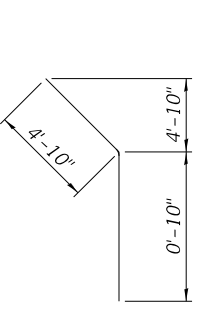


FIELD CUTTING DIAGRAM

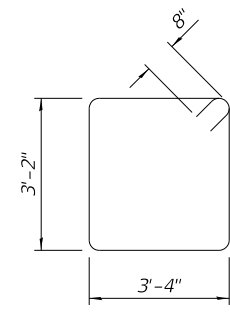
Order h1(E) and v2(E) full length. Cut as shown and use remainder of bars in opposite wing.



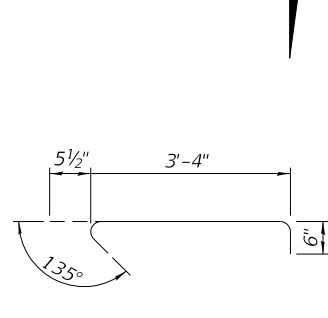
BAR v(E)
(Headed)



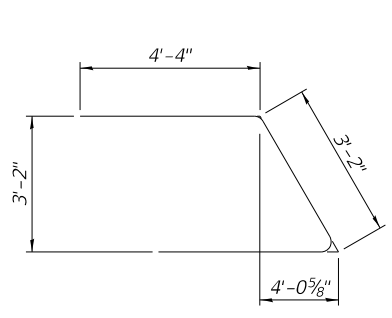
BAR h2(E)



BAR s(E)



BAR s1(E)



BAR u(E)

Notes:
 Pour steps monolithically with cap.
 Headed bars shall conform to ASTM A970 with threaded attachment; Class HA; and reinforcement bars conforming to ASTM A706. Cost included with Reinforcement Bars, Epoxy Coated.
 For details of piles see sheet 18 of 20.
 Bars indicated thus 20x3-#5 etc. indicates 20 lines of bars with 3 lengths per line.

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 IDFP NO. 184-001273

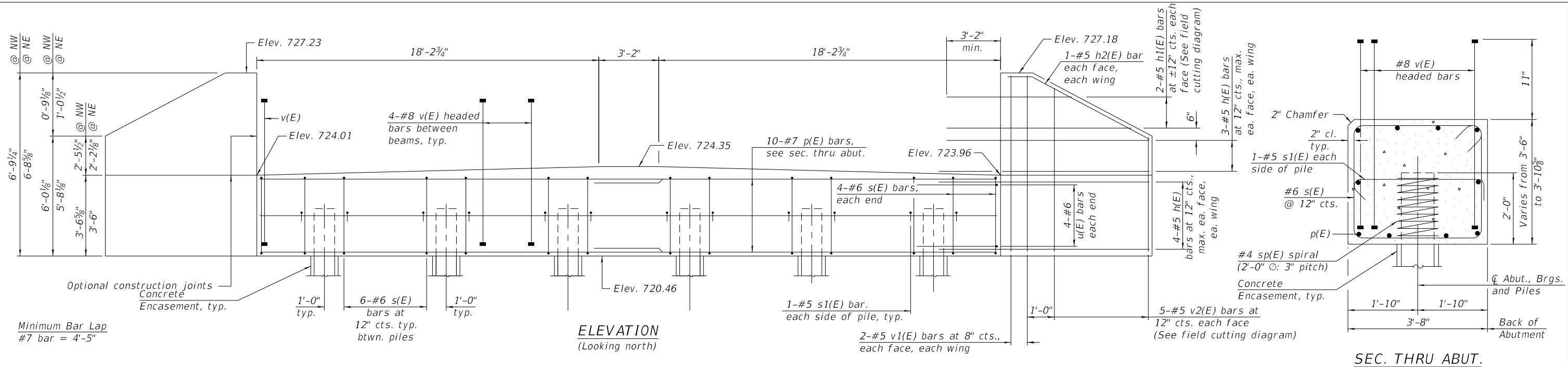
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CHECKED -	AJS	REVISD -	
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CITY OF WHEATON
 DUPAGE COUNTY, ILLINOIS

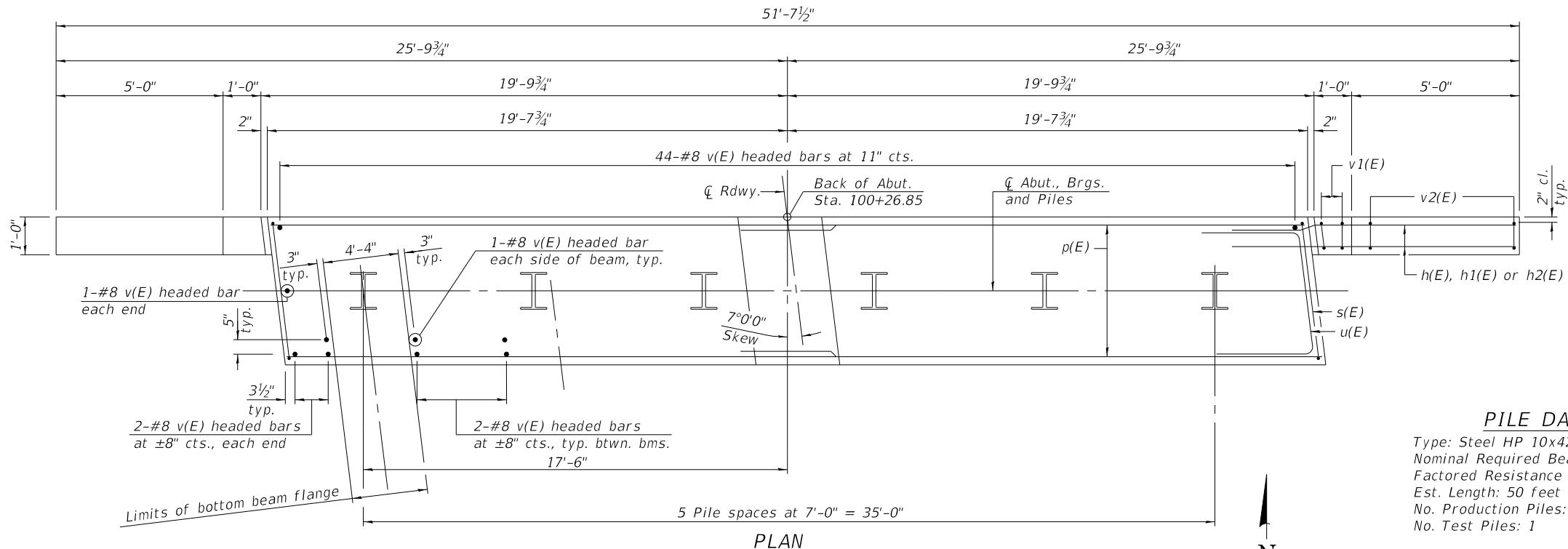
SOUTH ABUTMENT
 STRUCTURE NO. 022-7205

SHEET 16 OF 20 SHEETS

M.S. RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEK DRIVE	DUPAGE	47	31



SEC. THRU ABUT.



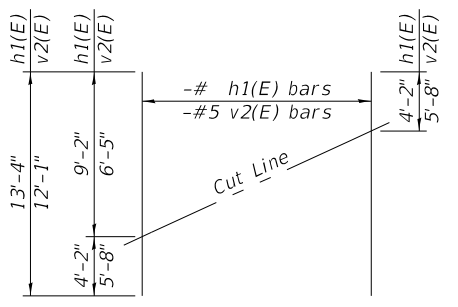
PLAN

BILL OF MATERIAL

Bar	No.	Size	Length	Shape
h(E)	12	#5	9'-2"	—
h1(E)	4	#5	13'-4"	—
h2(E)	4	#5	5'-9"	—
p(E)	20	#7	21'-8"	—
s(E)	38	#6	14'-4"	□
s1(E)	12	#5	4'-4"	—
u(E)	8	#6	11'-10"	—
v(E)	74	#8	4'-3"	—
v1(E)	8	#5	6'-5"	—
v2(E)	10	#5	12'-1"	—
sp(E)	6	#4	2'-0"	—
Structure Excavation		Cu. Yd.	67	
Concrete Structures		Cu. Yd.	22.4	
Concrete Encasement		Cu. Yd.	2	
Reinforcement Bars, Epoxy Coated		Pound	3122	
Furnishing Steel Piles HP10x42		Foot	260	
Driving Piles		Foot	250	
Test Pile Steel HP10x42		Each	1	
Pile Shoes		Each	6	
Granular Backfill for Structures		Cu. Yd.	14	
Geocomposite Wall Drain		Sq. Yd.	27	
Pipe Underdrain for Structures 4"		Foot	51	

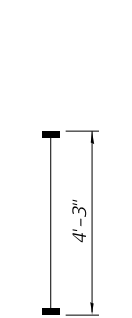
PILE DATA

Type: Steel HP 10x42 with Pile Shoes
 Nominal Required Bearing: 332 kips
 Factored Resistance Available:
 Est. Length: 50 feet
 No. Production Piles: 5
 No. Test Piles: 1

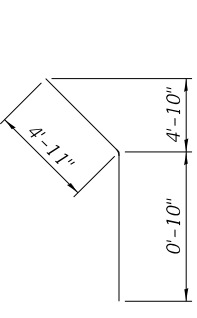


FIELD CUTTING DIAGRAM

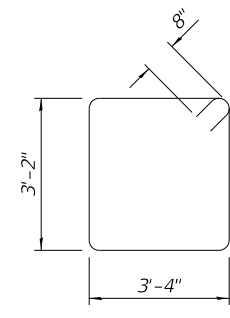
Order h1(E) and v2(E) full length. Cut as shown and use remainder of bars in opposite wing.



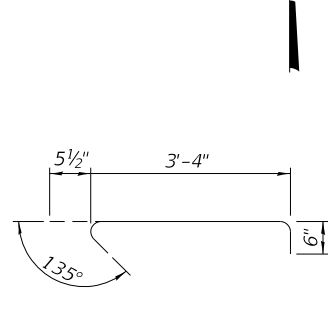
BAR v(E)
(Headed)



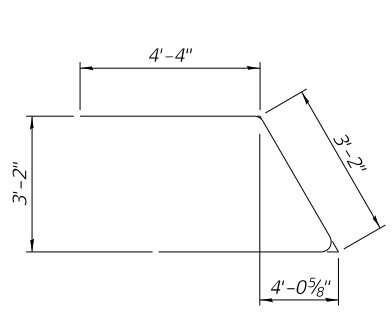
BAR h2(E)



BAR s(E)



BAR s1(E)



BAR u(E)

CITY OF WHEATON
 DUPAGE COUNTY, ILLINOIS

NORTH ABUTMENT
 STRUCTURE NO. 022-7205

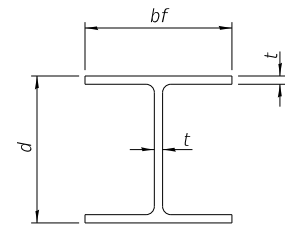
SHEET 17 OF 20 SHEETS

M.S. RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	32

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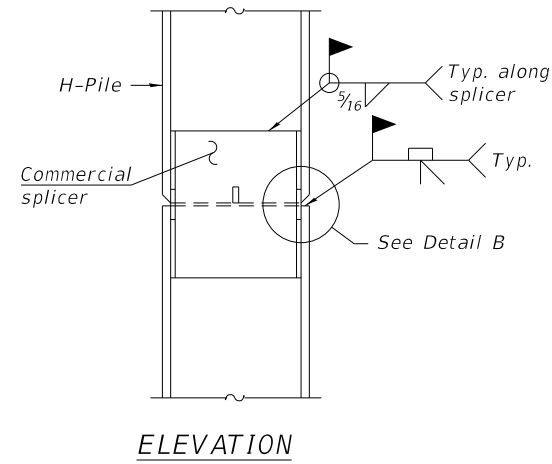
1170 SOUTH HOUBOLT ROAD
 JOLIET, ILLINOIS 60431
 (815) 744-4200
 IDFPFR NO. 184-001273

USER NAME	DESIGNED	REVISION
=	MJD	-
=	AJS	-
=	CJH	-
=	-	-

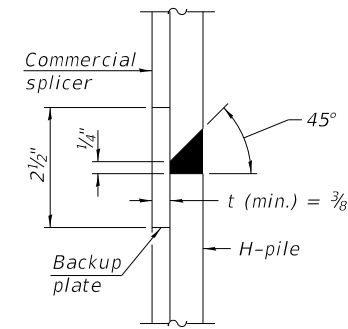


STEEL PILE TABLE

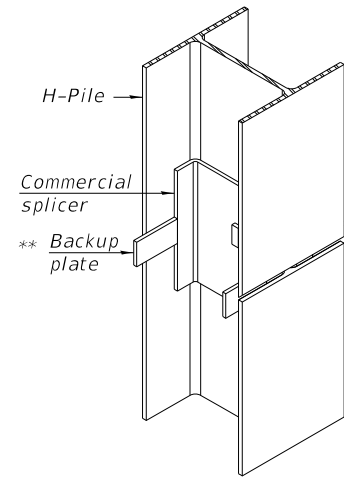
Designation	Depth d	Flange width bf	Web and Flange thickness t	Encasement diameter A
HP 14x117	14 1/4"	14 7/8"	1 3/16"	30"
x102	14"	14 3/4"	1 1/16"	30"
x89	13 7/8"	14 3/4"	5/8"	30"
x73	13 3/8"	14 3/8"	1/2"	30"
HP 12x84	12 1/4"	12 1/4"	1 1/16"	24"
x74	12 1/8"	12 1/4"	5/8"	24"
x63	12"	12 1/8"	1/2"	24"
x53	11 3/4"	12"	7/16"	24"
HP 10x57	10"	10 1/4"	9/16"	24"
x42	9 3/4"	10 1/8"	7/16"	24"
HP 8x36	8"	8 1/8"	7/16"	18"



ELEVATION

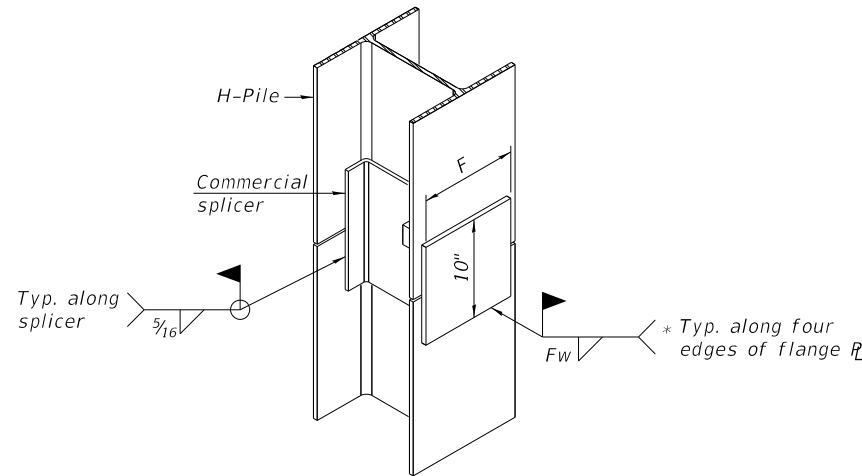


DETAIL "B"



ISOMETRIC VIEW

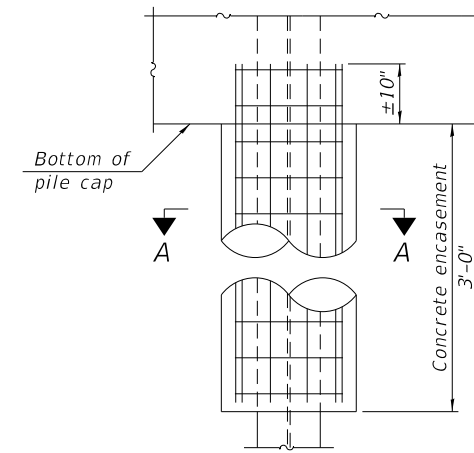
WELDED COMMERCIAL SPLICE



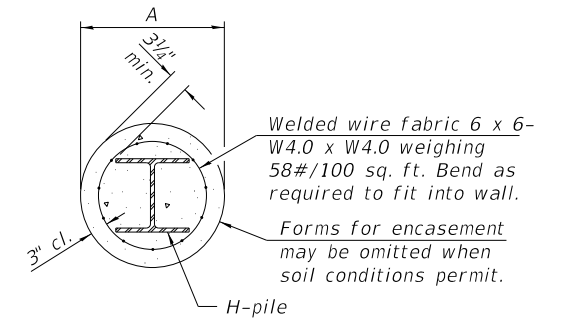
ISOMETRIC VIEW

WELDED COMMERCIAL SPLICE ALTERNATE

- * Interrupt welds 1/4" from end of web and/or each flange.
- ** Remove portions of backup plates that extend outside the flanges.
- *** Weld size per pile shoe manufacturer (5/16" min.).

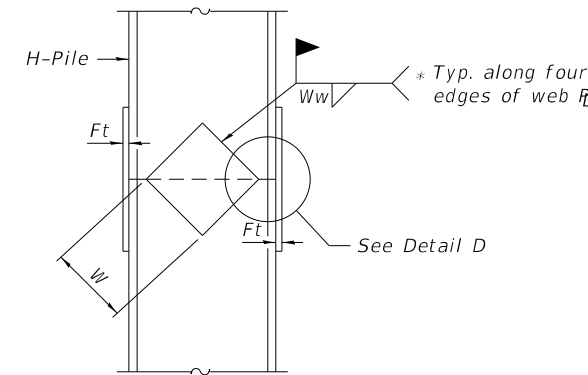


ELEVATION

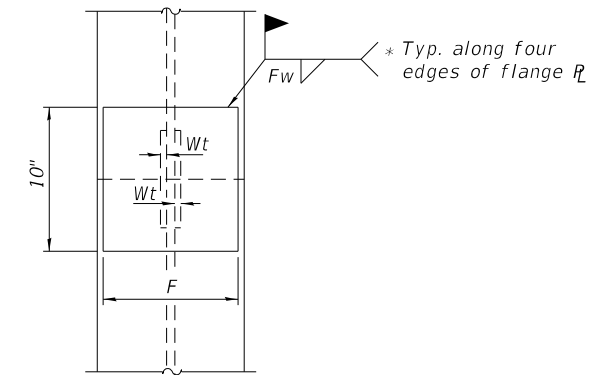


SECTION A-A

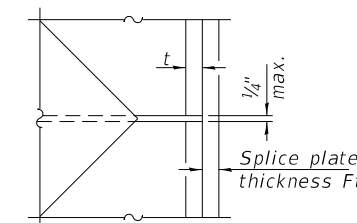
INDIVIDUAL PILE CONCRETE ENCASUREMENT (when specified)



ELEVATION



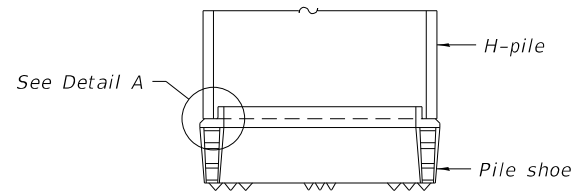
END VIEW



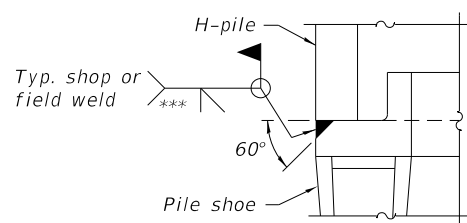
DETAIL D

WELDED PLATE FIELD SPLICE

Designation	F	Ft	Fw	W	Wt	Ww
HP 14x117	12 1/2"	1"	7/8"	7 3/4"	5/8"	1/2"
x102	12 1/2"	7/8"	3/4"	7 3/4"	5/8"	1/2"
x89	12 1/2"	3/4"	1 1/16"	7 3/4"	5/8"	1/2"
x73	12 1/2"	5/8"	9/16"	7 3/4"	5/8"	1/2"
HP 12x84	10"	7/8"	1 1/16"	6 1/2"	5/8"	1/2"
x74	10"	7/8"	1 1/16"	6 1/2"	5/8"	1/2"
x63	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
x53	10"	5/8"	1/2"	6 1/2"	1/2"	3/8"
HP 10x57	8"	3/4"	9/16"	5 1/4"	1/2"	3/8"
x42	8"	5/8"	9/16"	5 1/4"	1/2"	3/8"
HP 8x36	7"	5/8"	7/16"	4 1/4"	1/2"	3/8"



ELEVATION



DETAIL A

SHOE ATTACHMENT

Note:
The steel H-piles shall be according to AASHTO M270 Grade 50.

MODEL: Default
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F-HP 1-1-2020



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PLOT SCALE =	CHECKED - AJS	REVISED -
PLOT DATE =	DRAWN - CJH	REVISED -
	CHECKED -	REVISED -


CITY OF WHEATON
DUPAGE COUNTY, ILLINOIS

HP PILE DETAILS
STRUCTURE NO. 022-7205

SHEET 18 OF 20 SHEETS


M.S. RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	33

Soil Boring Log

 Construction & Geotechnical Material Testing, Inc. 60 Martin Lane, Elk Grove Village, Illinois 60007 Telephone (830) 505 1111 • Fax (830) 505 1110		Boring No:	B-01
		Date:	Monday, April 4, 2022
Soil Boring Prepared for: Mr. Sarang A. Lagvankar, P.E. City of Wheaton 303 W. Wesley Street Wheaton, Illinois 60187		Project:	Proposed Structure Replacement Creekside Drive, Wheaton, Illinois 60189
		Project No:	22G0214
		Boring Location:	See Boring Location Diagram
		Logged by:	L.S.H.
		Ground Elevation:	
Sheet 1 of 3			


Elevation	Depth	Soils	Soil / Rock Description	Sample Type & No. Depth Interval (ft) Recovery (in)	Blow Count	Moisture Content (%)	Unclassified Compressive Strength (TSF)	Notes & Test Results
0.0			Approximately 12" of Topsoil					Unclassified compressive strength of soil samples estimated using a calibrated penetrometer.
1.0			Silty CLAY, Trace Sand and Gravel, dark brown, stiff to very stiff (A-6-FILL)	SS-1 1.0' - 2.5' 6" Recovery	5 6 8	21.5	2.75	
2.0								
3.0								
4.0			Silty CLAY, Trace Organics, Sand and Gravel, dark brown, very soft to stiff (A-6-FILL)	SS-2 3.5' - 5.0' 12" Recovery	5 9 12	22.5	1.25	Dry Density: 3.5' - 5.0' = 100.3 lbs/ft ³
5.0								
6.0			Saturated	SS-3 6.0' - 7.5' 18" Recovery	3 3 5	30.7	1.0	Dry Density: 6.0' - 7.5' = 91.3 lbs/ft ³ Organic Content: 6.0' - 7.5' = 4.8%
7.0								
8.0			Saturated	SS-4 8.5' - 10.0' 3" Recovery	8 8 5	33.6	<0.25	Organic Content: 8.5' - 10.0' = 6.3%
9.0								
10.0			Silty LOAM, Trace Sand and Gravel, gray, medium dense (A-4)	SS-5 11.0' - 12.5' 18" Recovery	5 8 8	19.8	-	
11.0								
12.0			SANDY CLAY LOAM, Trace Gravel, gray, dense (A-4)	SS-6 13.5' - 15.0' 18" Recovery	8 10 14	17.6	--	Rimac Spring Tester: 13.5' - 15.0' = 0.29 TSF
13.0								
14.0			Sandy GRAVEL, gray, medium dense (A-1)	SS-7 18.5' - 20.0' 1" Recovery	8 10 12	13.2	-	
15.0								
16.0								
17.0								
18.0								
19.0								
20.0								
Drilling Contractor: CGMTCDS				Water Level (Ft.)				
Drilling Method: 3/4" O.D. H.S.A. Split Spoon Sampling				During Drilling: 10 feet				
Drilling Equipment: CME-ATV Drill Rig				Immediately After Drilling: 20 feet				
REVIEWED BY: NPW								

Soil Boring Log

 Construction & Geotechnical Material Testing, Inc. 60 Martin Lane, Elk Grove Village, Illinois 60007 Telephone (830) 505 1111 • Fax (830) 505 1110		Boring No:	B-01
		Date:	Monday, April 4, 2022
Soil Boring Prepared for: Mr. Sarang A. Lagvankar, P.E. City of Wheaton 303 W. Wesley Street Wheaton, Illinois 60187		Project:	Proposed Structure Replacement Creekside Drive, Wheaton, Illinois 60189
		Project No:	22G0214
		Boring Location:	See Boring Location Diagram
		Logged by:	L.S.H.
		Ground Elevation:	
Sheet 2 of 3			

Elevation	Depth	Soils	Soil / Rock Description	Sample Type & No. Depth Interval (ft) Recovery (in)	Blow Count	Moisture Content (%)	Unclassified Compressive Strength (TSF)	Notes & Test Results
20.0			Sandy GRAVEL, gray, medium dense (A-1)					Unclassified compressive strength of soil samples estimated using a calibrated penetrometer.
21.0								
22.0			Silty CLAY LOAM, Trace Sand and Gravel, gray, medium dense to very dense (A-6)	SS-8 23.5' - 25.0' 18" Recovery	2 5 8	19.5	--	Rimac Spring Tester: 23.5' - 25.0' = 0.29 TSF
23.0								
24.0								
25.0								
26.0								
27.0			Saturated	SS-9 28.5' - 30.0' 18" Recovery	6 12 8	20.1	--	Rimac Spring Tester: 28.5' - 30.0' = 0.29 TSF
28.0								
29.0			SANDY CLAY LOAM, Trace Gravel, gray, dense (A-4)	SS-10 33.5' - 35.0' 18" Recovery	22 34 34	23.7	--	Rimac Spring Tester: 33.5' - 35.0' = 0.29 TSF
30.0								
31.0			Sandy GRAVEL, gray, medium dense (A-1)	SS-11 38.5' - 40.0' 12" Recovery	10 16 8	13.1	--	Rimac Spring Tester: 38.5' - 40.0' = 0.29 TSF
32.0								
33.0								
34.0								
35.0								
36.0								
37.0								
38.0								
39.0								
40.0								
Drilling Contractor: CGMTCDS				Water Level (Ft.)				
Drilling Method: 3/4" O.D. H.S.A. Split Spoon Sampling				During Drilling: 10 feet				
Drilling Equipment: CME-ATV Drill Rig				Immediately After Drilling: 20 feet				
REVIEWED BY: NPW								

Soil Boring Log

 Construction & Geotechnical Material Testing, Inc. 60 Martin Lane, Elk Grove Village, Illinois 60007 Telephone (830) 505 1111 • Fax (830) 505 1110		Boring No:	B-01
		Date:	Monday, April 4, 2022
Soil Boring Prepared for: Mr. Sarang A. Lagvankar, P.E. City of Wheaton 303 W. Wesley Street Wheaton, Illinois 60187		Project:	Proposed Structure Replacement Creekside Drive, Wheaton, Illinois 60189
		Project No:	22G0214
		Boring Location:	See Boring Location Diagram
		Logged by:	L.S.H.
		Ground Elevation:	
Sheet 3 of 3			

Elevation	Depth	Soils	Soil / Rock Description	Sample Type & No. Depth Interval (ft) Recovery (in)	Blow Count	Moisture Content (%)	Unclassified Compressive Strength (TSF)	Notes & Test Results
40.0			Silty CLAY LOAM, Trace Sand and Gravel, gray, medium dense to very dense (A-6)					Unclassified compressive strength of soil samples estimated using a calibrated penetrometer.
41.0								
42.0								
43.0			Saturated	SS-12 43.5' - 45.0' 18" Recovery	10 22 28	15.2	--	Rimac Spring Tester: 43.5' - 45.0' = 0.29 TSF
44.0								
45.0			Saturated	SS-13 48.5' - 50.0' No Recovery	50 -	-	-	Spoon and Auger Refusal at 50 Feet: Underground Obstruction (Possible Cobbles, Boulders or Bedrock) Encountered During The Advancement Of the Split Spoon Sampler and Auger
46.0								
47.0			SPOON AND AUGER REFUSAL at 50 Feet END of BORING at 50 Feet					
48.0								
49.0								
50.0								
51.0								
52.0								
53.0								
54.0								
55.0								
56.0								
57.0								
58.0								
59.0								
60.0								
Drilling Contractor: CGMTCDS				Water Level (Ft.)				
Drilling Method: 3/4" O.D. H.S.A. Split Spoon Sampling				During Drilling: 10 feet				
Drilling Equipment: CME-ATV Drill Rig				Immediately After Drilling: 20 feet				
REVIEWED BY: NPW								

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1170 SOUTH HOUBOLT ROAD
 JOLIET, ILLINOIS 60431
 (815) 744-4200
 IDFPR NO. 184-001273

USER NAME =	DESIGNED -	MJD	REVISED -	
PLOT SCALE =	CHECKED -	AJS	REVISED -	
PLOT DATE =	DRAWN -	CJH	REVISED -	
	CHECKED -		REVISED -	

**CITY OF WHEATON
 DUPAGE COUNTY, ILLINOIS**

**SOIL BORING (SHEET 1 OF 2)
 STRUCTURE NO. 022-7205**

SHEET 19 OF 20 SHEETS

M.S. RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	34

Soil Boring Log

Construction & Geotechnical Material Testing, Inc.
 60 Martin Lane, Elk Grove Village, Illinois 60007
 Telephone (630) 595-1111 • Fax (630) 595-1110

Boring No: **B-02**
 Date: **Monday, April 4, 2022**
 Project: **Proposed Structure Replacement**
Creekside Drive, Wheaton, Illinois 60189

Soil Boring Prepared for:
Mr. Sarang A. Lagvankar, P.E.
 City of Wheaton
 303 W. Wesley Street
 Wheaton, Illinois 60187

Project No: **22G0214**
 Boring Location: **See Boring Location Diagram**

Logged by: **L.S.H.**
 Ground Elevation: _____

Elevation	Depth	Strata	Soil / Rock Description	Sample Type & No Depth Interval (ft) Recovery (in)	Blow Count	Moisture Content (%)	Unclassified Compressive Strength (TSF)	Notes & Test Results
0.0			Approximately 20' of Topsoil					Unclassified compressive strength of soil samples estimated using a calibrated penetrometer.
1.0			Sandy CLAY, Trace Gravel, brown, firm to very stiff (A-6-FILL)	SS-1 1.0' - 2.5' 10" Recovery	3	8.8	2.0	
2.0								
3.0								
4.0			Saturated	SS-2 3.5' - 5.0' 5" Recovery	3	13.6	0.75	
5.0								
6.0			Sandy Clayey GRAVEL, brown, medium dense (A-1)	SS-3 6.0' - 7.5' 5" Recovery	2	18.9	0.5	
7.0								
8.0								
9.0			Saturated	SS-4 8.5' - 10.0' 7" Recovery	3	10.8	-	
10.0								
11.0			Saturated					
12.0								
13.0								
14.0			Saturated	SS-5 13.5' - 15.0' 4" Recovery	3	20.4	-	
15.0								
16.0			SAND, Trace Gravel, gray, medium dense to very dense (A-2)					
17.0								
18.0								
19.0				SS-6 18.5' - 20.0' 8" Recovery	27	17	8.8	-
20.0								

Drilling Contractor: **CGMTCDS** Water Level (Ft.): _____

Drilling Method: **3/4" O.D. H.S.A. Split Spoon Sampling** During Drilling: **6 feet**

Drilling Equipment: **CME-45C Truck-Mounted Drill Rig** Immediately After Drilling: **8 feet**

REVIEWED BY: **NPW**

Soil Boring Log

Construction & Geotechnical Material Testing, Inc.
 60 Martin Lane, Elk Grove Village, Illinois 60007
 Telephone (630) 595-1111 • Fax (630) 595-1110

Boring No: **B-02**
 Date: **Monday, April 4, 2022**
 Project: **Proposed Structure Replacement**
Creekside Drive, Wheaton, Illinois 60189

Soil Boring Prepared for:
Mr. Sarang A. Lagvankar, P.E.
 City of Wheaton
 303 W. Wesley Street
 Wheaton, Illinois 60187

Project No: **22G0214**
 Boring Location: **See Boring Location Diagram**

Logged by: **L.S.H.**
 Ground Elevation: _____

Elevation	Depth	Strata	Soil / Rock Description	Sample Type & No Depth Interval (ft) Recovery (in)	Blow Count	Moisture Content (%)	Unclassified Compressive Strength (TSF)	Notes & Test Results
20.0			SAND, Trace Gravel, gray, medium dense to very dense (A-2)					Unclassified compressive strength of soil samples estimated using a calibrated penetrometer.
21.0								
22.0								
23.0			Saturated	SS-7 23.5' - 25.0' 13" Recovery	37	40	20.9	-
24.0								
25.0			SILTY CLAY LOAM, Trace Sand and Gravel, gray, medium dense to dense (A-4)					Rimac Spring Tester: 28.5' - 30.0' = 0.29 TSF
26.0								
27.0								
28.0								
29.0			Saturated	SS-8 28.5' - 30.0' 16" Recovery	7	9	18.9	-
30.0								
31.0			Saturated					Rimac Spring Tester: 33.5' - 35.0' = 0.29 TSF
32.0								
33.0								
34.0			Saturated	SS-9 33.5' - 35.0' 16" Recovery	7	6	19.8	-
35.0								
36.0			Saturated					Rimac Spring Tester: 38.5' - 40.0' = 0.29 TSF
37.0								
38.0								
39.0				SS-10 38.5' - 40.0' 17" Recovery	8	9	24.9	
40.0								

Drilling Contractor: **CGMTCDS** Water Level (Ft.): _____

Drilling Method: **3/4" O.D. H.S.A. Split Spoon Sampling** During Drilling: **6 feet**

Drilling Equipment: **CME-45C Truck-Mounted Drill Rig** Immediately After Drilling: **8 feet**

REVIEWED BY: **NPW**

Soil Boring Log

Construction & Geotechnical Material Testing, Inc.
 60 Martin Lane, Elk Grove Village, Illinois 60007
 Telephone (630) 595-1111 • Fax (630) 595-1110

Boring No: **B-02**
 Date: **Monday, April 4, 2022**
 Project: **Proposed Structure Replacement**
Creekside Drive, Wheaton, Illinois 60189

Soil Boring Prepared for:
Mr. Sarang A. Lagvankar, P.E.
 City of Wheaton
 303 W. Wesley Street
 Wheaton, Illinois 60187

Project No: **22G0214**
 Boring Location: **See Boring Location Diagram**

Logged by: **L.S.H.**
 Ground Elevation: _____

Elevation	Depth	Strata	Soil / Rock Description	Sample Type & No Depth Interval (ft) Recovery (in)	Blow Count	Moisture Content (%)	Unclassified Compressive Strength (TSF)	Notes & Test Results
40.0			SILTY CLAY LOAM, Trace Sand and Gravel, gray, medium dense to dense (A-4)					Unclassified compressive strength of soil samples estimated using a calibrated penetrometer.
41.0								
42.0								
43.0			Saturated	SS-11 43.5' - 45.0' 18" Recovery	10	19	14.2	-
44.0								Rimac Spring Tester: 43.5' - 45.0' = 0.29 TSF
45.0			Saturated					Spoon and Auger Refusal at 50 Feet: Underground Obstruction (Possible Cobbles, Boulders or Bedrock) Encountered During The Advancement Of the Split Spoon Sampler and Auger
46.0								
47.0								
48.0			Saturated	SS-12 48.5' - 50.0' No Recovery	50	-	-	-
49.0								
50.0			SPOON AND AUGER REFUSAL at 50 Feet END of BORING at 50 Feet					
51.0			Saturated					
52.0								
53.0								
54.0								
55.0								
56.0								
57.0								
58.0								
59.0								
60.0								

Drilling Contractor: **CGMTCDS** Water Level (Ft.): _____

Drilling Method: **3/4" O.D. H.S.A. Split Spoon Sampling** During Drilling: **6 feet**

Drilling Equipment: **CME-ATV Drill Rig** Immediately After Drilling: **8 feet**

REVIEWED BY: **NPW**

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1/31/2023 11:03:53 AM

STRAND ASSOCIATES
 1170 SOUTH HOUBOLT ROAD
 JOUETT, ILLINOIS 60431
 (615) 744-4200
 DFPNR NO. 184-001273

USER NAME =	DESIGNED - MJD	REVISED -
PLOT SCALE =	CHECKED - AJS	REVISED -
PLOT DATE =	DRAWN - CJH	REVISED -
	CHECKED -	REVISED -

**CITY OF WHEATON
 DUPAGE COUNTY, ILLINOIS**

**SOIL BORINGS (SHEET 2 OF 2)
 STRUCTURE NO. 022-7205**

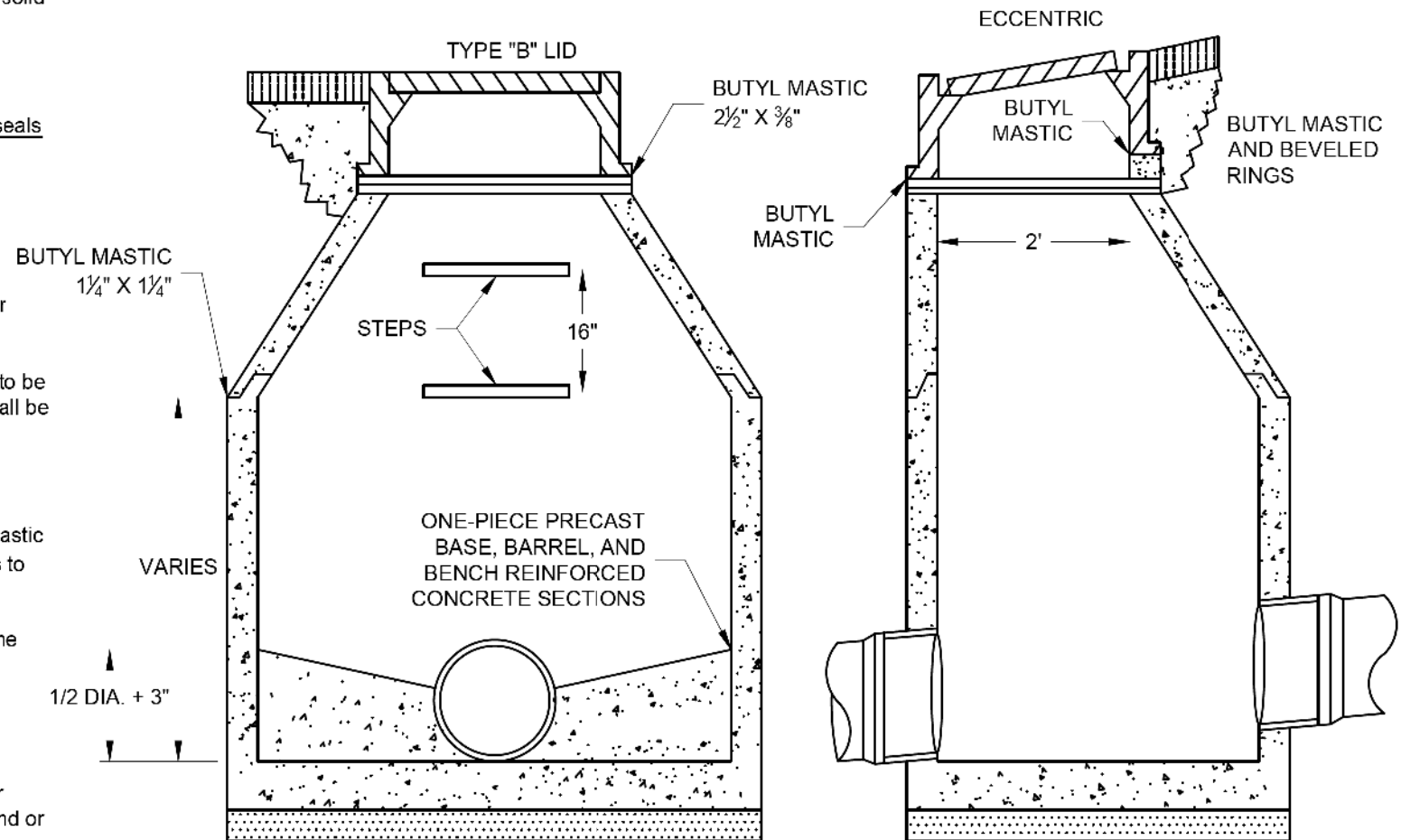
SHEET 20 OF 20 SHEETS

M.S. RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	35

SANITARY MANHOLE

NOTES:

1. Sanitary Manhole castings
 - A. In paved and parkway areas Neenah R-1772 (380 lbs.) with a solid lid.
 - B. In backlot areas Neenah R-1772 (325 lbs.).
2. All frames and lids must have concealed pick holes and gasketed self-sealing features. "O" ring seals are not permitted. Flat gaskets "T" seals are required, and frame must accommodate chimney seal.
3. All lids must be stamped "Sanitary Manhole".
4. Manholes to have one piece integral base and barrel section.
5. Manholes to be 4' diameter for sewers 18" or less, and 5' diameter for sewers larger than 18" and less than 48".
6. Manholes 4' in diameter which exceed a depth of 4' shall have steps to be coated with polypropylene plastic, "Press Fit" type installation. Steps shall be 16" apart on center.
7. All Manholes to have chimney seal collars.
8. All joints between barrel sections shall be sealed with flexible butyl mastic material 1/4" wide by 1/4" thick. All joints between riser rings and frames to be sealed with flexible butyl mastic material 2 1/2" wide 3/8" thick.
9. External or internal chimney seals must be installed between the dome and frame.
10. All connections to be by preinstalled synthetic rubber sleeve with stainless steel clamps, non shear-type.
11. When shimming to grade is required in paved areas, beveled rubber adjustment rings and butyl mastic shall be used. No shimming of any kind or or mortar layers are allowed.
13. The maximum amount of adjustment for frame elevation is 8" with 2 riser rings. Concrete adjusting rings shall have a minimum thickness of 2". Rubber adjusting rings shall total no more than 3". Beveled riser rings shall be used in cases where a difference in elevation exists. Metal riser rings are not allowed and bricks shall not be used for shimming the frame on the structure.
14. When an eccentric cone is used:
 - A. If the Manhole is of 4' dia. then the cone shall be of 2'-6" in height.
 - B. If the Manhole is of 5' dia. then the cone shall be of 3'-9" in height



CITY OF WHEATON	
DESIGN DETAILS AND STANDARDS	
FIGURE # 16	
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	DRAWN - CJH	REVISED -	
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PLOT DATE = 1/31/2023	DATE -	REVISED -	

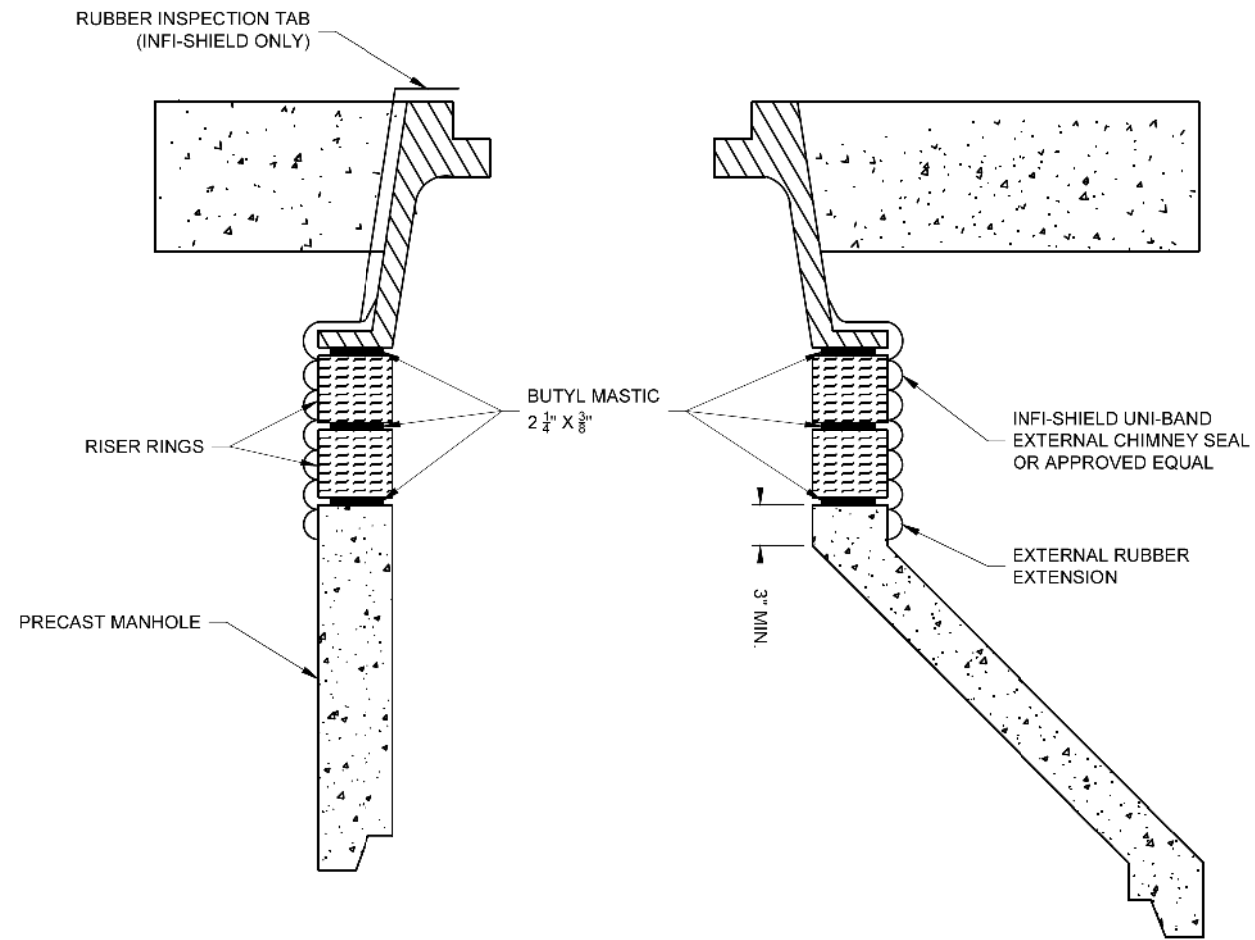
**CITY OF WHEATON
DUPAGE COUNTY, ILLINOIS**

**CREEKSIDE DRIVE OVER SPRING BROOK NO. 1
CITY STANDARDS**

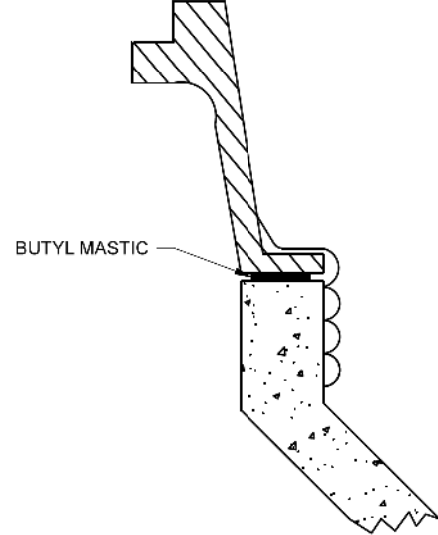
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MS RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	36

SANITARY MANHOLE EXTERNAL CHIMNEY SEAL



NARROW EXTERNAL RUBBER SEAL



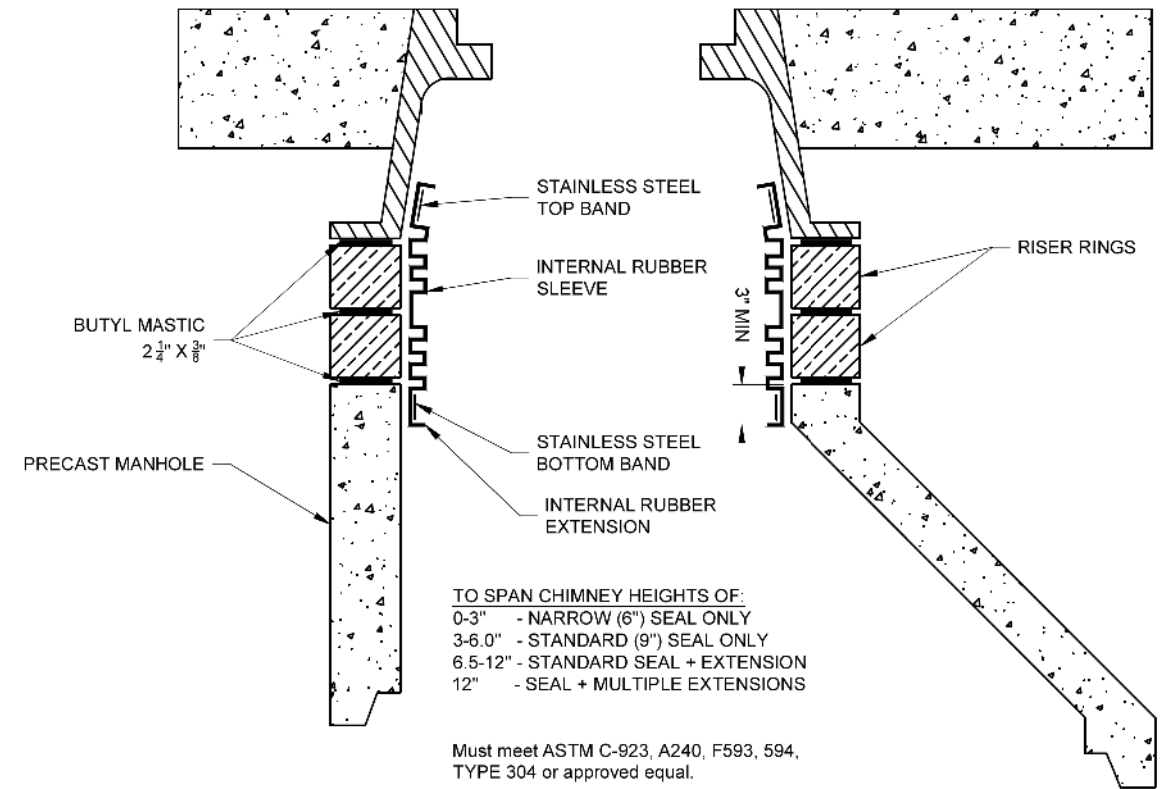
TO SPAN CHIMNEY HEIGHTS OF:
 0-3" - NARROW (6") SEAL ONLY
 3-6.0" - STANDARD (9") SEAL ONLY
 6.5-12" - STANDARD SEAL + EXTENSION
 12" - SEAL + MULTIPLE EXTENSIONS

Must meet ASTM C-923, A240, F593, 594, TYPE 304 or approved equal.

Maximum adjustment of 8" using 2 rings.

CITY OF WHEATON	
DESIGN DETAILS AND STANDARDS	
FIGURE # 17	
DATE: 12-07-20	FILE NAME: Sanitary Manhole External Chimney Seal.dwg

SANITARY SEWER INTERNAL CHIMNEY SEAL



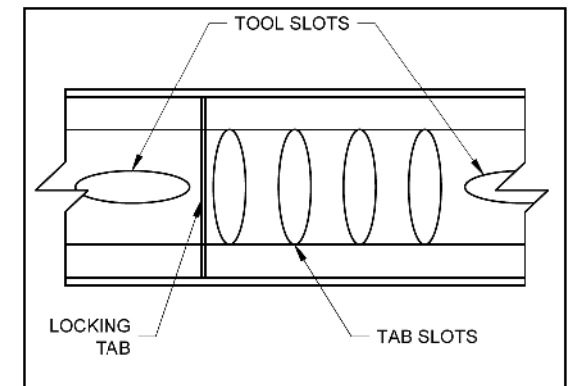
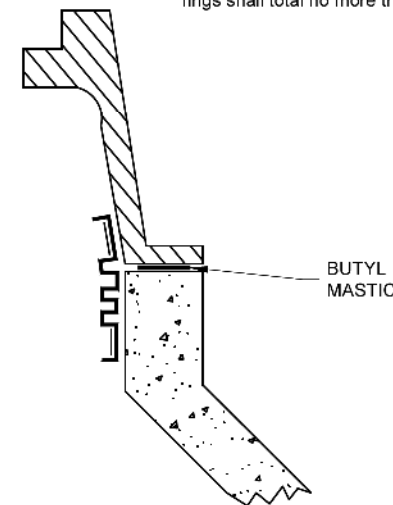
TO SPAN CHIMNEY HEIGHTS OF:
 0-3" - NARROW (6") SEAL ONLY
 3-6.0" - STANDARD (9") SEAL ONLY
 6.5-12" - STANDARD SEAL + EXTENSION
 12" - SEAL + MULTIPLE EXTENSIONS

Must meet ASTM C-923, A240, F593, 594, TYPE 304 or approved equal.

NARROW INTERNAL RUBBER SEAL

The maximum amount of adjustment for frame elevation is 8" with 2 riser rings. Concrete adjusting rings shall have a minimum thickness of 2". Rubber adjusting rings shall total no more than 3".

STAINLESS STEEL ADJUSTMENT BAND



CITY OF WHEATON	
DESIGN DETAILS AND STANDARDS	
FIGURE # 18	
DATE: 12-07-20	FILE NAME: Sanitary Manhole Internal Chimney Seal.dwg

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PLOT SCALE = 2,000' / in.	DRAWN - CJH	REVISED -
PLOT DATE = 1/31/2023	CHECKED - AJS	REVISED -
	DATE -	REVISED -

CITY OF WHEATON
 DUPAGE COUNTY, ILLINOIS

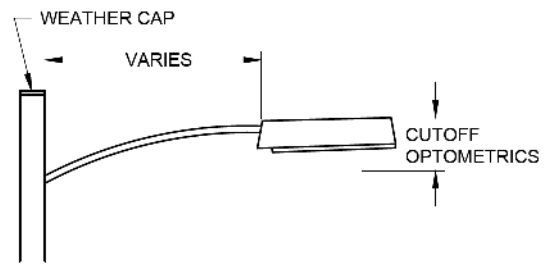
CREEKSIDE DRIVE OVER SPRING BROOK NO. 1
 CITY STANDARDS

SCALE: NTS SHEET 2 OF 8 SHEETS STA. N/A TO STA. N/A

MS RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	37

TYPICAL STREET LIGHT DETAIL

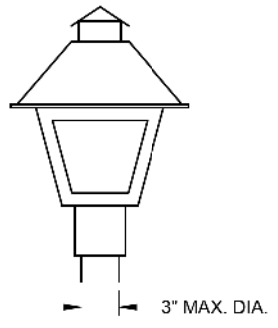
HIGHWAY



LUMINAIRE
150/250/310 watt HPSV as required McGraw Edison (Unidor series) UUI284, UUI265 or approved equal.

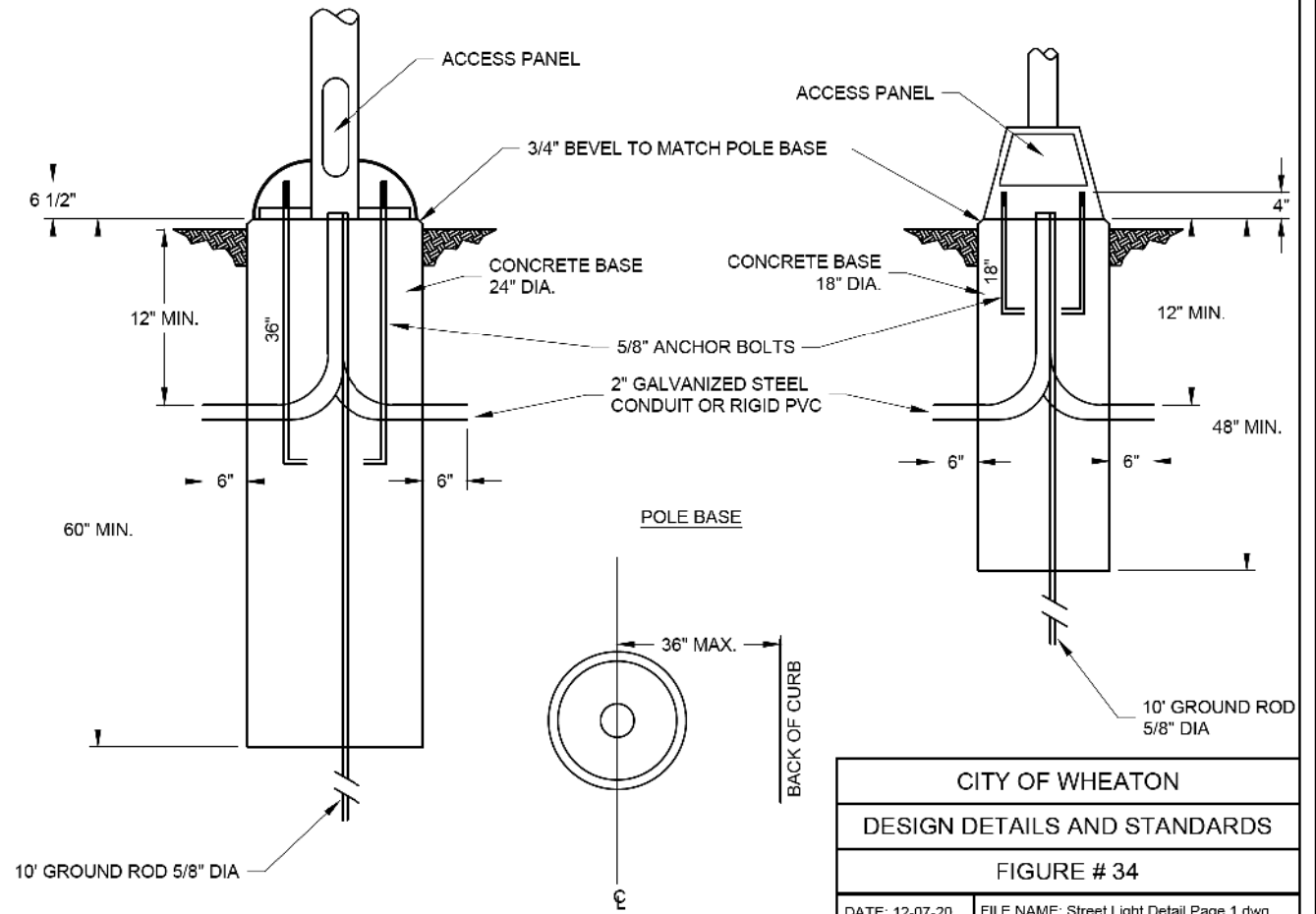
POLE AND MAST ARM
Mast arm mounted street light
-mounting height = 25.0'
Hapco 21-293 plain base (single arm) or approved equal.

RESIDENTIAL



LUMINAIRE
40 watt LED
Cyclone :CL41T4-FLAF-GAL-5-40W-4K-120-EA1-DAP-F2AP-CP4091-R30-RAL9005TX or approved equal.

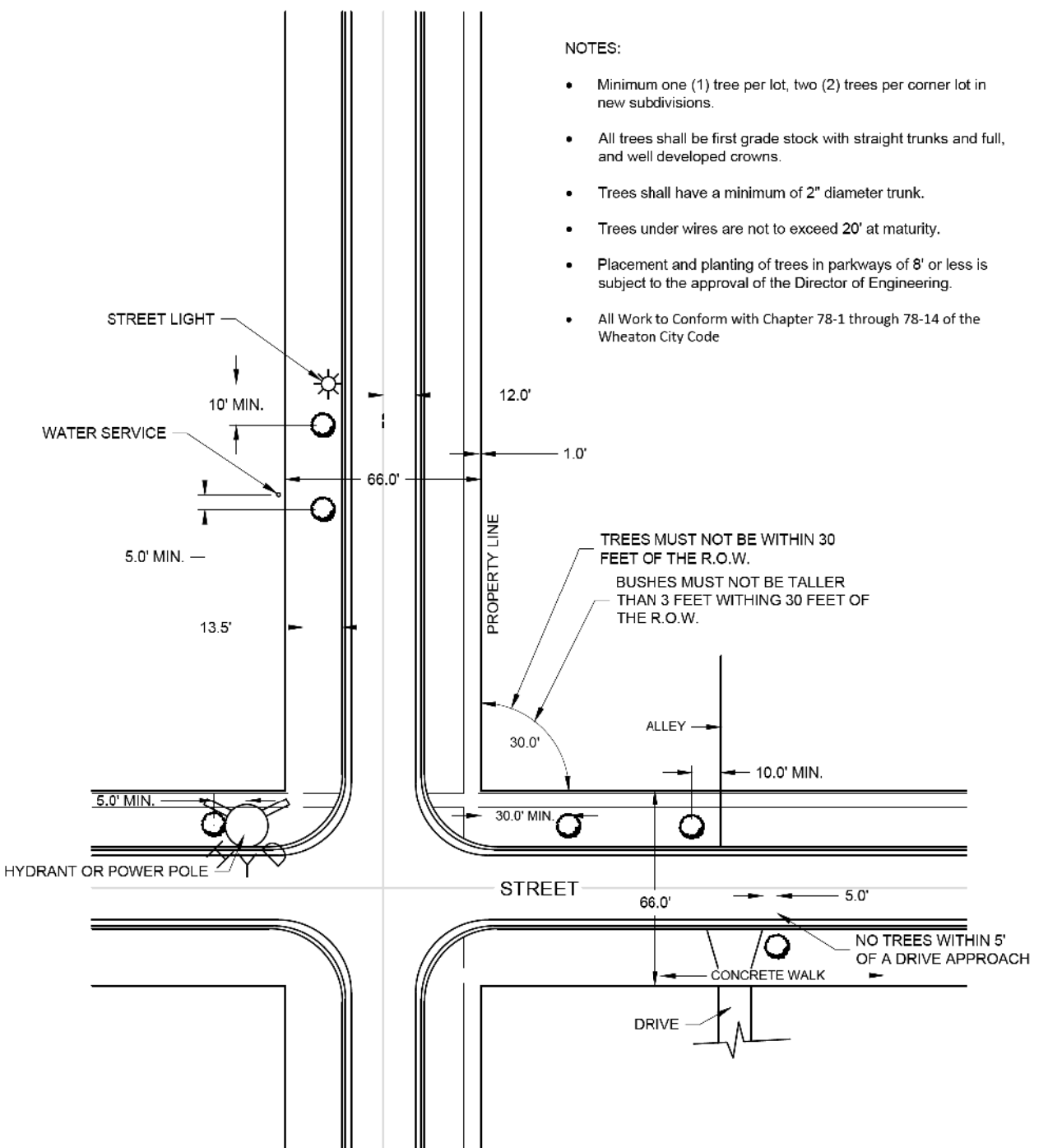
POST OR POLE
Post mounted street light
-mounting height = 14.0'
Hapco 77-005 McGraw Edison PA-4002, or approved equal.



CITY OF WHEATON
DESIGN DETAILS AND STANDARDS
FIGURE # 34
DATE: 12-07-20 FILE NAME: Street Light Detail Page 1.dwg

PARKWAY TREE PLANTING STANDARDS

For more information reference "Wheaton City Code" Section 62-294; Chapter 78, 70-208
Wheaton Zoning Ordinance, Sec 6.5.3.



- NOTES:
- Minimum one (1) tree per lot, two (2) trees per corner lot in new subdivisions.
 - All trees shall be first grade stock with straight trunks and full, and well developed crowns.
 - Trees shall have a minimum of 2" diameter trunk.
 - Trees under wires are not to exceed 20' at maturity.
 - Placement and planting of trees in parkways of 8' or less is subject to the approval of the Director of Engineering.
 - All Work to Conform with Chapter 78-1 through 78-14 of the Wheaton City Code

CITY OF WHEATON
DESIGN DETAILS AND STANDARDS
FIGURE # 43
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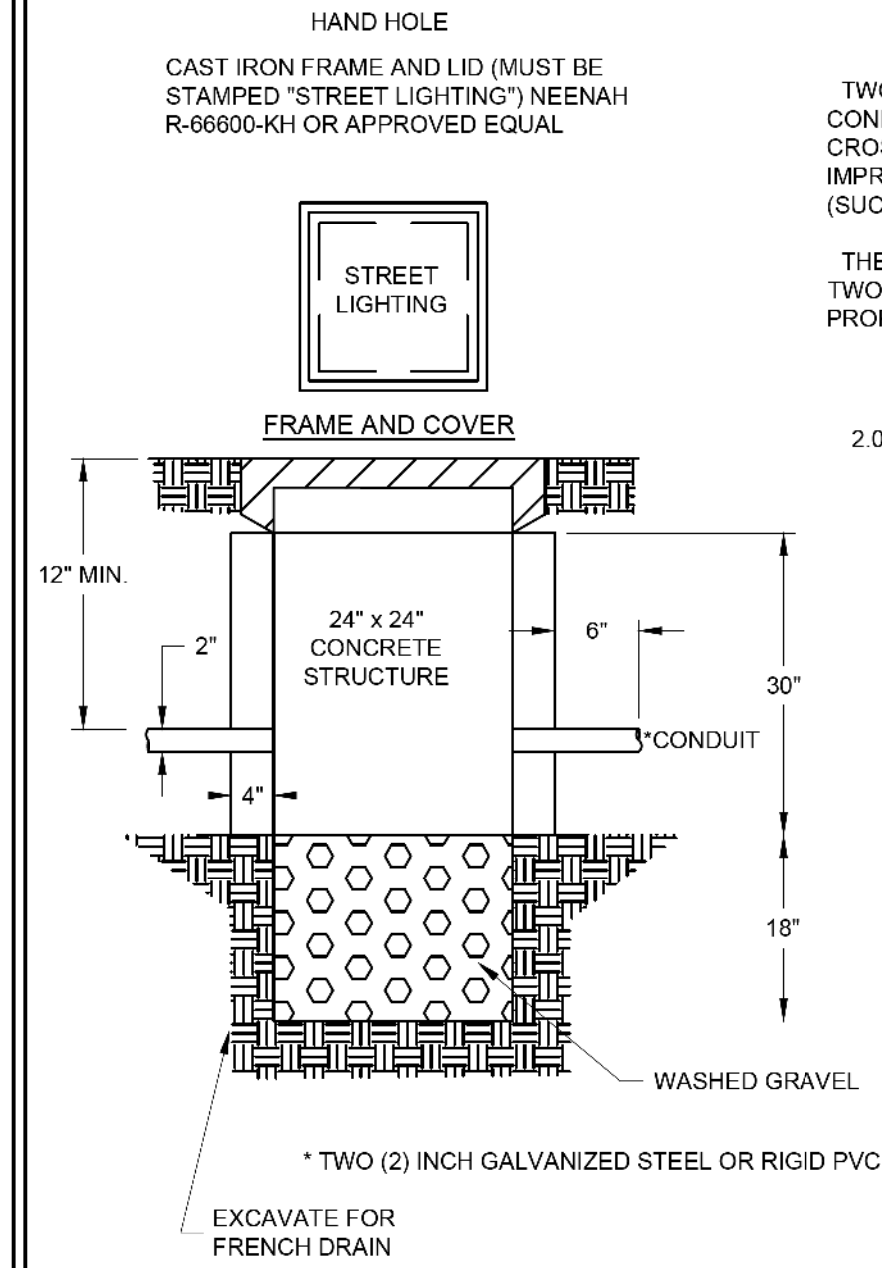
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	DATE -	REVISED -

CITY OF WHEATON
DUPAGE COUNTY, ILLINOIS

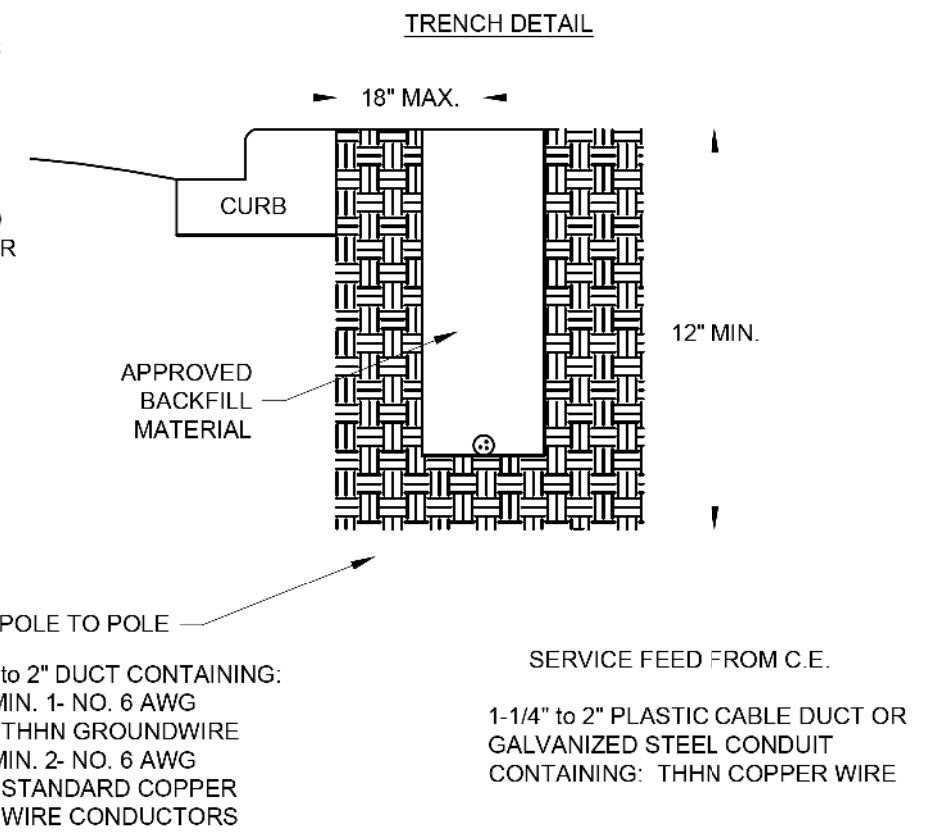
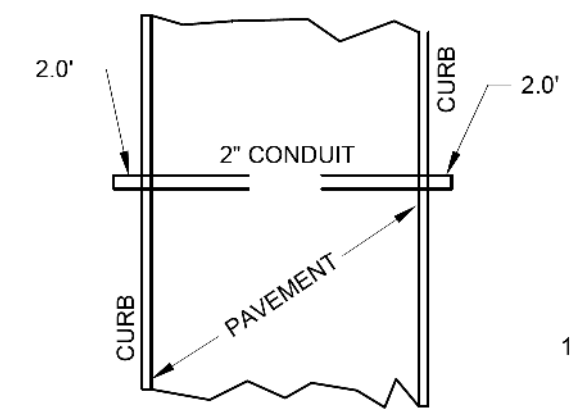
CREEKSIDE DRIVE OVER SPRING BROOK NO. 1 CITY STANDARDS			
SCALE: NTS	SHEET 3	OF 8 SHEETS	STA. N/A TO STA. N/A

MS RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	38

TYPICAL STREET LIGHT DETAIL



TYPICAL STREET CROSSING
 TWO (2) INCH GALVANIZED STEEL OR RIGID PVC CONDUIT WILL BE USED WHERE CABLE DUCT CROSSES UNDER ANY EXISTING OR PROPOSED IMPROVEMENTS. (SUCH AS STREETS, DRIVES, WALKS, ETC.)
 THE GALVANIZED STEEL CONDUIT WILL EXTEND TWO (2) FEET MINIMUM BEYOND ANY EXISTING OR PROPOSED IMPROVEMENTS.



NOTES: ALL CONDUCTORS MUST MEET MINIMUM THHN SPECIFICATIONS

CITY OF WHEATON	
DESIGN DETAILS AND STANDARDS	
FIGURE # 35	
DATE: 12-07-20	FILE NAME: Street Light Detail Page 2.dwg

MODEL: D:\dwg\...
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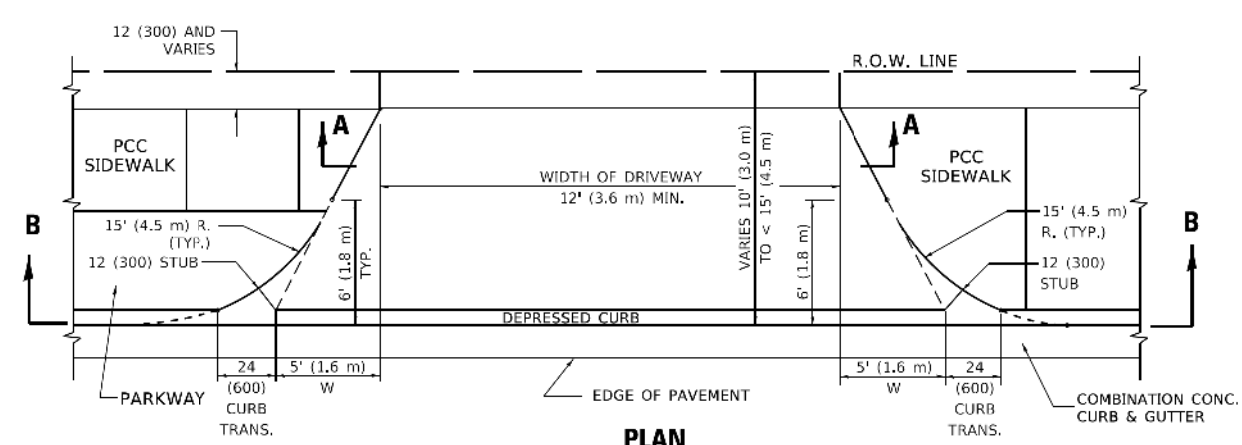
USER NAME = AlexSc	DESIGNED - AMS	REVISED -
PLOT SCALE = 2,000' / in.	DRAWN - CJH	REVISED -
PLOT DATE = 1/31/2023	CHECKED - AJS	REVISED -
	DATE -	REVISED -

**CITY OF WHEATON
 DUPAGE COUNTY, ILLINOIS**

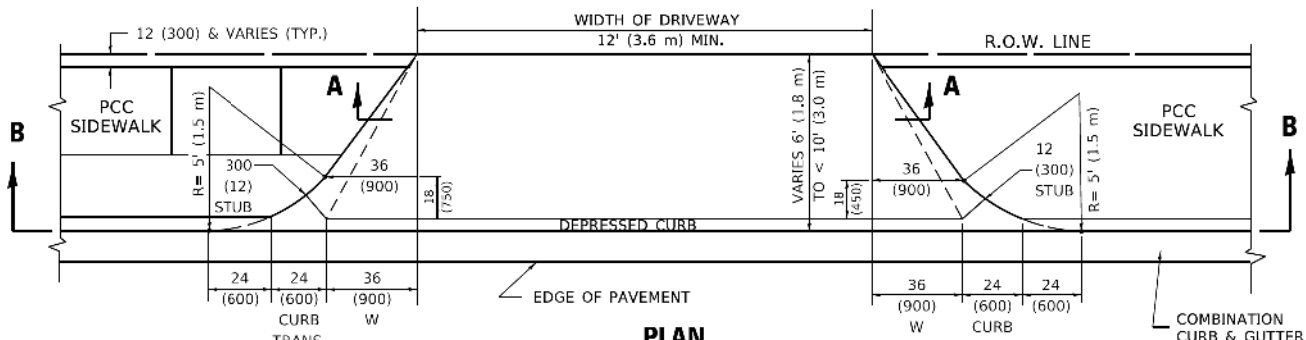
**CREEKSIDE DRIVE OVER SPRING BROOK NO. 1
 CITY STANDARDS**

SCALE: NTS SHEET 4 OF 8 SHEETS STA. N/A TO STA. N/A

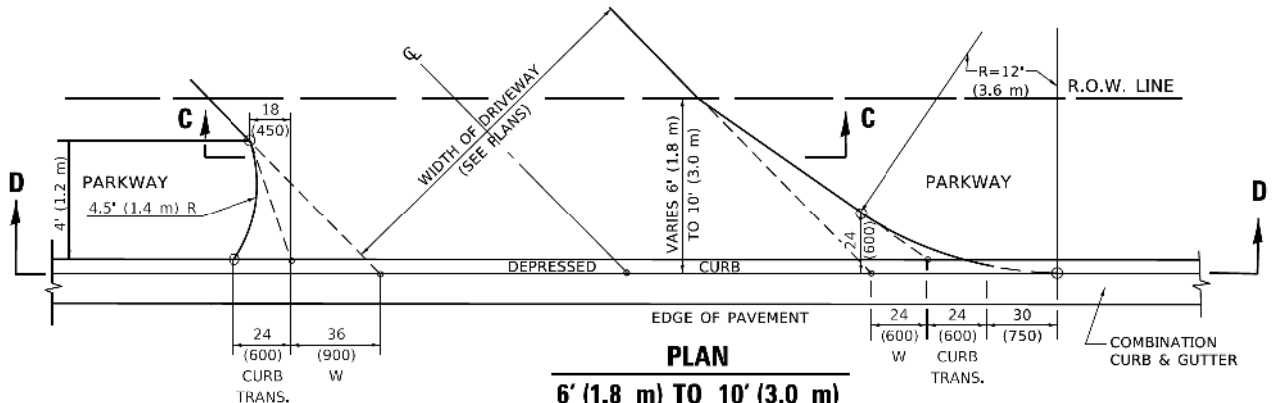
MS RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	39



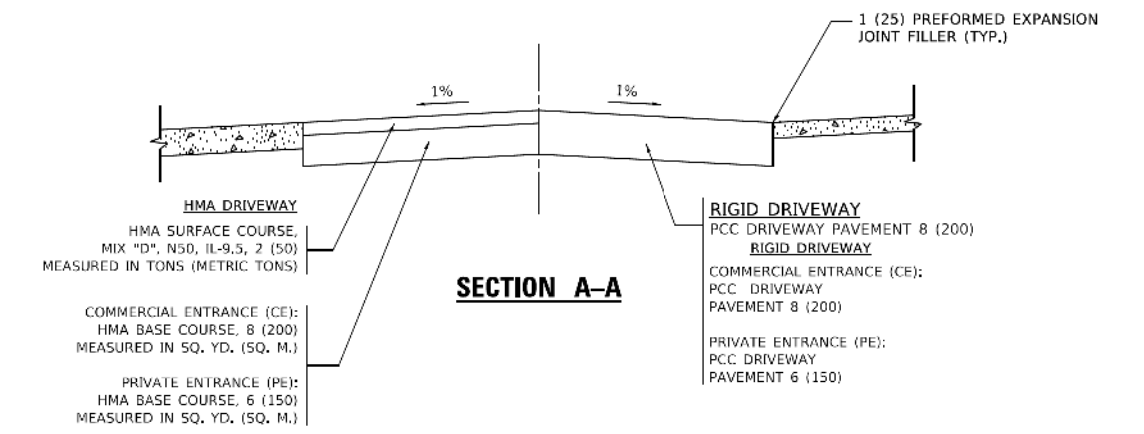
PLAN
10' (3.0 m) TO < 15' (4.5 m)



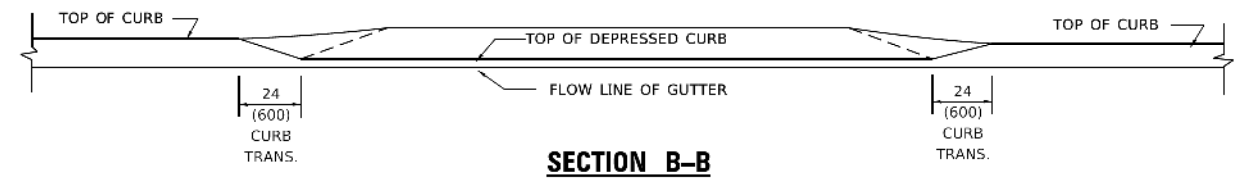
PLAN
6' (1.8 m) TO < 10' (3.0 m)



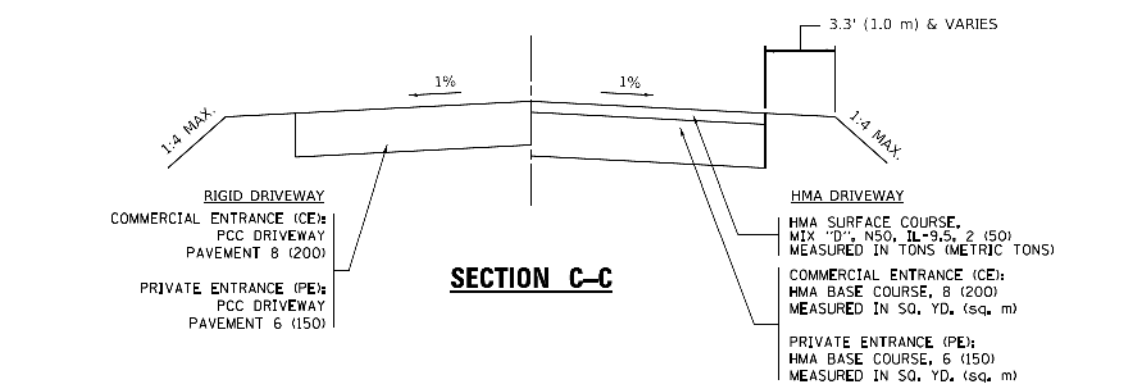
PLAN
6' (1.8 m) TO 10' (3.0 m)



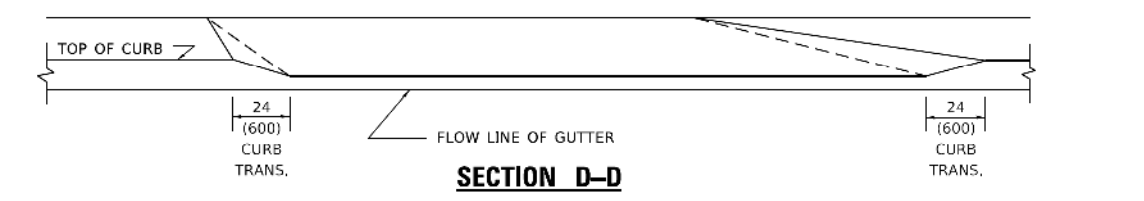
SECTION A-A



SECTION B-B



SECTION C-C



SECTION D-D

GENERAL NOTES

- DRIVEWAY SLOPES, LOCATIONS, & GEOMETRIC LAYOUT SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE "HANDBOOK FOR POLICY ON PERMITS FOR ACCESS DRIVEWAYS TO STATE HIGHWAYS". FOR FURTHER LAYOUT REQUIREMENTS, REFER TO ILLUSTRATION 10 IN THE PERMIT HANDBOOK. WHERE SIDEWALKS EXIST, DRIVEWAYS SHALL BE REPLACED WITH RIGID PAVEMENT. WHERE NO SIDEWALKS EXIST, DRIVEWAYS SHALL BE REPLACED IN KIND. SIDEWALK CROSS SLOPE THRU DRIVEWAY AREA TO BE A MAXIMUM OF 1:50.
- WHEN THE DISTANCE BETWEEN R.O.W. AND THE BACK OF CURB IS EQUAL TO OR LESS THAN 8' (2.4 m), THE PCC SIDEWALK SHALL EXTEND TO THE BACK OF CURB.
- "W" VARIES FROM 36 (900) TO 5' (1.5 m) PROPORTIONAL TO THE LENGTH (L), FROM 6' (1.8 m) TO 10' (3 m).

ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE NOTED.

USER NAME = dmancheit	DESIGNED - R. SHAH	REVISED - R. BORO 01-01-07	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	DRIVEWAY DETAILS		F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
PLOT SCALE = 1/20.0000' / in.	CHECKED -	REVISED - R. BORO 09-06-11		DISTANCE BETWEEN ROW AND FACE OF CURB < 15' (4.5m)						
PLOT DATE = 2/27/2022	DATE - 11-06-95	REVISED - K. SMITH 08-27-19		SCALE: NONE	SHEET 1 OF 1 SHEETS	STA.	TO STA.	BD400-02 (BD-02) CONTRACT NO.		
		REVISED - K. SMITH 02-01-22						ILLINOIS FED. AID PROJECT		

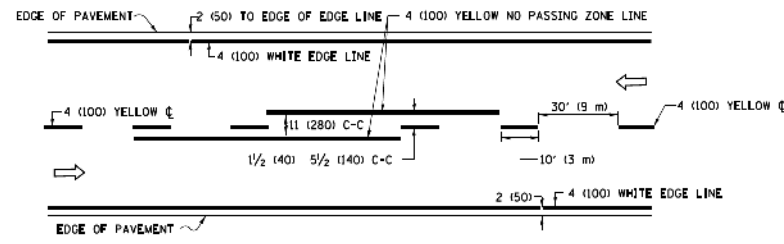
USER NAME = AlexSc	DESIGNED - AMS	REVISED -
	DRAWN - CJH	REVISED -
PLOT SCALE = 2.0000' / in.	CHECKED - AJS	REVISED -
PLOT DATE = 1/31/2023	DATE -	REVISED -

**CITY OF WHEATON
DUPAGE COUNTY, ILLINOIS**

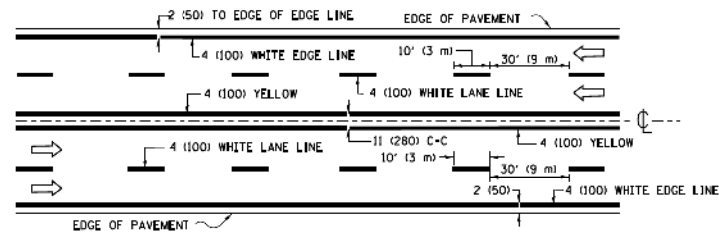
**CREEKSIDE DRIVE OVER SPRING BROOK NO. 1
DISTRICT STANDARDS**

SCALE: NTS	SHEET 5 OF 8 SHEETS	STA. N/A TO STA. N/A	MS RTE. 4065	MARKED ROUTE CREEKSIDE DRIVE	COUNTY DUPAGE	TOTAL SHEETS 47	SHEET NO. 40
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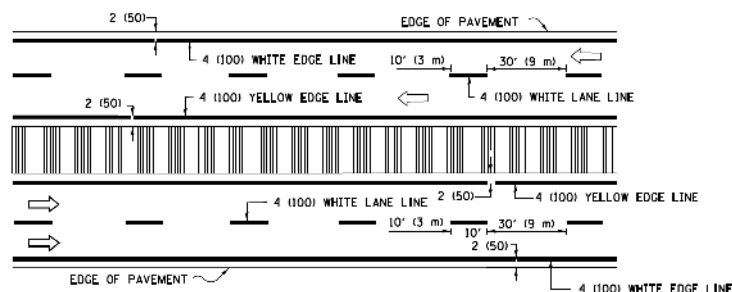
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2-LANE ROADWAY

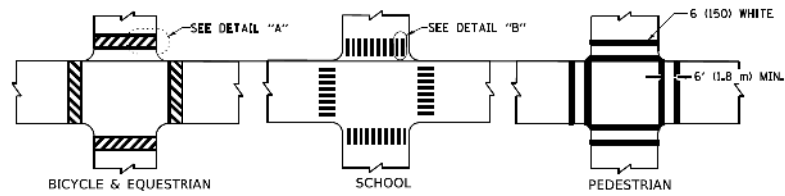


MULTI-LANE UNDIVIDED



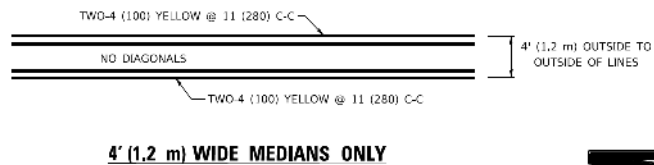
MULTI-LANE DIVIDED WITH MEDIAN

TYPICAL LANE AND EDGE LINE MARKING

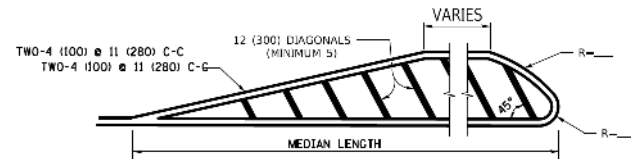


TYPICAL CROSSWALK MARKING

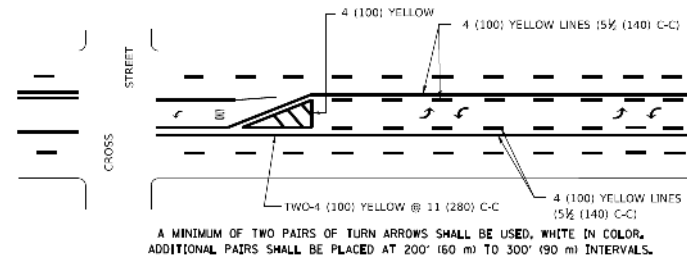
* MARKINGS SHALL BE INSTALLED PARALLEL TO THE CENTERLINE OF THE ROAD WHICH IT CROSSES



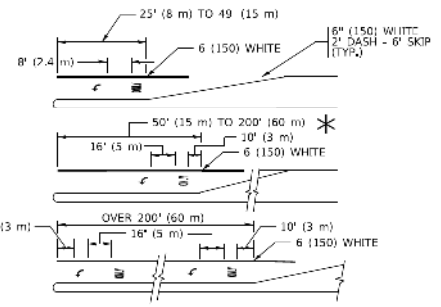
4' (1.2 m) WIDE MEDIANS ONLY



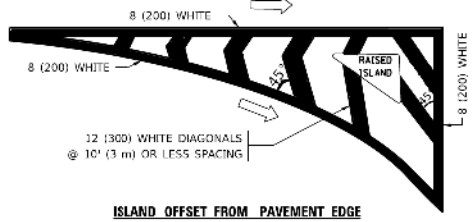
MEDIANS OVER 4' (1.2 m) WIDE



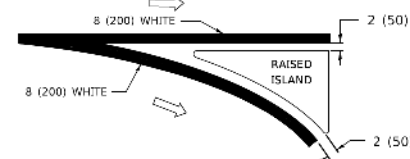
MEDIAN WITH TWO-WAY LEFT TURN LANE TYPICAL PAINTED MEDIAN MARKING



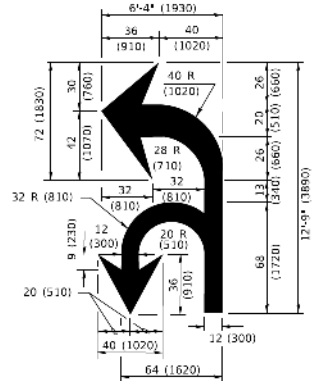
TYPICAL LEFT (OR RIGHT) TURN LANE TYPICAL TURN LANE MARKING



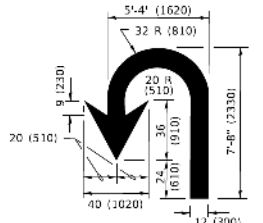
ISLAND OFFSET FROM PAVEMENT EDGE



ISLAND AT PAVEMENT EDGE TYPICAL ISLAND MARKING



COMBINATION LEFT AND U-TURN



U-TURN

LANE REDUCTION TRANSITION

* LANE REDUCTION ARROWS REQUIRED AT SPEEDS OF 45 MPH OR GREATER OR WHEN SPECIFIED IN PLANS.

D(FT)	SPEED LIMIT
345	30
425	35
500	40
580	45
665	50
750	55

TYPE OF MARKING	WIDTH OF LINE	PATTERN	COLOR	SPACING / REMARKS
CENTERLINE ON 2 LANE PAVEMENT	4 (100)	SKIP-DASH	YELLOW	10' (3 m) LINE WITH 30' (9 m) SPACE
CENTERLINE ON MULTI-LANE UNDIVIDED PAVEMENT	2 @ 4 (100)	SOLID	YELLOW	11 (280) C-C
NO PASSING ZONE LINES: FOR ONE DIRECTION FOR BOTH DIRECTIONS	4 (100) 2 @ 4 (100)	SOLID SOLID	YELLOW YELLOW	5' (140) C-C FROM SKIP-DASH CENTERLINE 11 (280) C-C OMIT SKIP-DASH CENTERLINE BETWEEN
LANE LINES	4 (100) 5 (125) ON FREEWAYS	SKIP-DASH SKIP-DASH	WHITE WHITE	10' (3 m) LINE WITH 30' (9 m) SPACE
DOTTED LINES (EXTENSIONS OF CENTER, LANE OR TURN LANE MARKINGS)	SAME AS LINE BEING EXTENDED	SKIP-DASH	SAME AS LINE BEING EXTENDED	2' (600) LINE WITH 6' (1.8 m) SPACE
EDGE LINES	4 (100)	SOLID	YELLOW-LEFT WHITE-RIGHT	OUTLINE MEDIANS IN YELLOW
TURN LANE MARKINGS	6 (150) LINE: FULL SIZE LETTERS & SYMBOLS 8' (2.4m)	SOLID	WHITE	SEE TYPICAL TURN LANE MARKING DETAIL
TWO WAY LEFT TURN MARKING	2 @ 4 (100) EACH DIRECTION 8' (2.4m) LEFT ARROW	SKIP-DASH AND SOLID IN PAIRS	YELLOW WHITE	10' (3 m) LINE WITH 30' (9 m) SPACE FOR SKIP-DASH; 5' (140) C-C BETWEEN SOLID LINE AND SKIP-DASH LINE SEE TYPICAL TWO-WAY LEFT TURN MARKING DETAIL
CROSSWALK LINES (PEDESTRIAN) A. DIAGONALS (BIKE & EQUESTRIAN) B. LONGITUDINAL BARS (SCHOOL)	2 @ 6 (150) 12 (300) @ 45° 12 (300) @ 90°	SOLID SOLID SOLID	WHITE WHITE WHITE	NOT LESS THAN 6' (1.8 m) APART 2' (600) APART SEE TYPICAL CROSSWALK MARKING DETAILS.
STOP LINES	24 (600)	SOLID	WHITE	PLACE 4' (1.2 m) IN ADVANCE OF AND PARALLEL TO CROSSWALK. IF PRESENT, OTHERWISE, PLACE AT DESIRED STOPPING POINT. PARALLEL TO CROSSROAD CENTERLINE, WHERE POSSIBLE
PAINTED MEDIANS	2 @ 4 (100) WITH 12 (300) DIAGONALS @ 45° NO DIAGONALS USED FOR 4' (1.2 m) WIDE MEDIANS	SOLID	YELLOW: TWO WAY TRAFFIC WHITE: ONE WAY TRAFFIC	11 (280) C-C FOR THE DOUBLE LINE SEE TYPICAL PAINTED MEDIAN MARKING.
GORE MARKING AND CHANNELIZING LINES	8 (200) WITH 12 (300) DIAGONALS @ 45°	SOLID	WHITE	DIAGONALS: 15' (4.5 m) C-C (LESS THAN 30MPH (50 km/h)) 20' (6 m) C-C 30MPH (50 km/h) TO 45MPH (70 km/h) 30' (9 m) C-C (OVER 45MPH (70 km/h))
RAILROAD CROSSING	24 (600) TRANSVERSE LINES; "RR" IS 6' (1.8 m) LETTERS; 16 (400) LINE FOR "X"	SOLID	WHITE	SEE STATE STANDARD 780001 AREA OF: "R" = 3.6 SQ. FT. (0.33 m ²) EACH "X" = 54.0 SQ. FT. (5.0 m ²)
SHOULDER DIAGONALS (REQUIRED FOR SHOULDERS > 8')	12 (300) @ 45°	SOLID	WHITE - RIGHT YELLOW - LEFT	50' (15 m) C-C (LESS THAN 30MPH (50 km/h)) 75' (25 m) C-C (30 MPH (50 km/h) TO 45MPH (70 km/h)) 150' (45 m) C-C (OVER 45MPH (70 km/h))
U TURN ARROW	SEE DETAIL	SOLID	WHITE	16.3 SF
2 ARROW COMBINATION LEFT AND U TURN	SEE DETAIL	SOLID	WHITE	30.4 SF

FOR FURTHER DETAILS ON PAVEMENT MARKING REFER TO STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION AND STATE STANDARD 780001.

All dimensions are in inches (millimeters) unless otherwise shown.

MODEL Defaul... FILE Name: S:\011100-11991122\013\Drawings\CADD\micross-SA\CAD_Sheets\1122013-3-28-Chy\01-Detail.dgn

USER NAME = faconrj	DESIGNED - EVERS	REVISED - C. JUCIUS 09-09-09	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	DISTRICT ONE TYPICAL PAVEMENT MARKINGS	F.A. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
PLOT SCALE = 30.0000' / in.	DRAWN -	REVISED - C. JUCIUS 07-01-13			SCALE: NONE	SHEET 1 OF 2 SHEETS STA.	TC-13	ILLINOIS	CONTRACT NO.	
PLOT DATE = 1/4/2019	CHECKED -	REVISED - C. JUCIUS 12-21-15								
	DATE - 03-19-90	REVISED - C. JUCIUS 04-12-16								

USER NAME = AlexSc	DESIGNED - AMS	REVISED -	CITY OF WHEATON DUPAGE COUNTY, ILLINOIS	CREEKSIDE DRIVE OVER SPRING BROOK NO. 1 DISTRICT STANDARDS	MS RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.	
PLOT SCALE = 2,000.0' / in.	DRAWN - CJH	REVISED -			SCALE: NTS	SHEET 6 OF 8 SHEETS STA.	4065	CREEKSIDE DRIVE	DUPAGE	47
PLOT DATE = 1/31/2023	CHECKED - AJS	REVISED -								
	DATE -	REVISED -								

ROUTE MARKERS

FOR U.S. ROUTES
M1-40-2424

FOR ILLINOIS ROUTES
M1-50-2424

R.R. UNMARKED ROUTES
SPECIAL 24" x 18" VARIABLE
4" BLACK LETTERS ON WHITE
REFLECTIVE BACKGROUND

ARROWS SIGNS

M5-1L-2115

M5-1R-2115

M6-1-2115

M6-2-2115

M6-3-2115

CARDINAL DIRECTION & DETOUR SIGNS

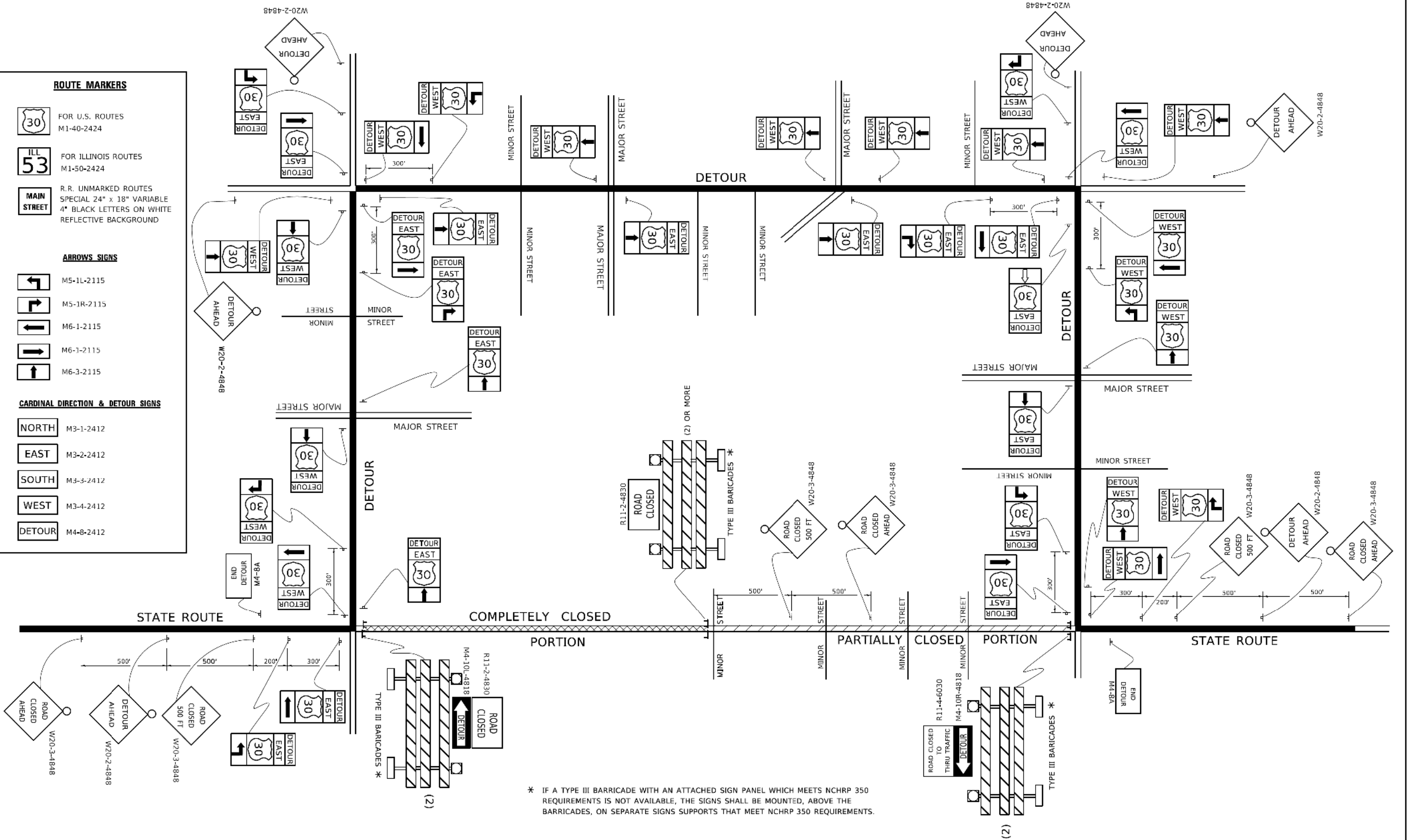
NORTH M3-1-2412

EAST M3-2-2412

SOUTH M3-3-2412

WEST M3-4-2412

DETOUR M4-8-2412



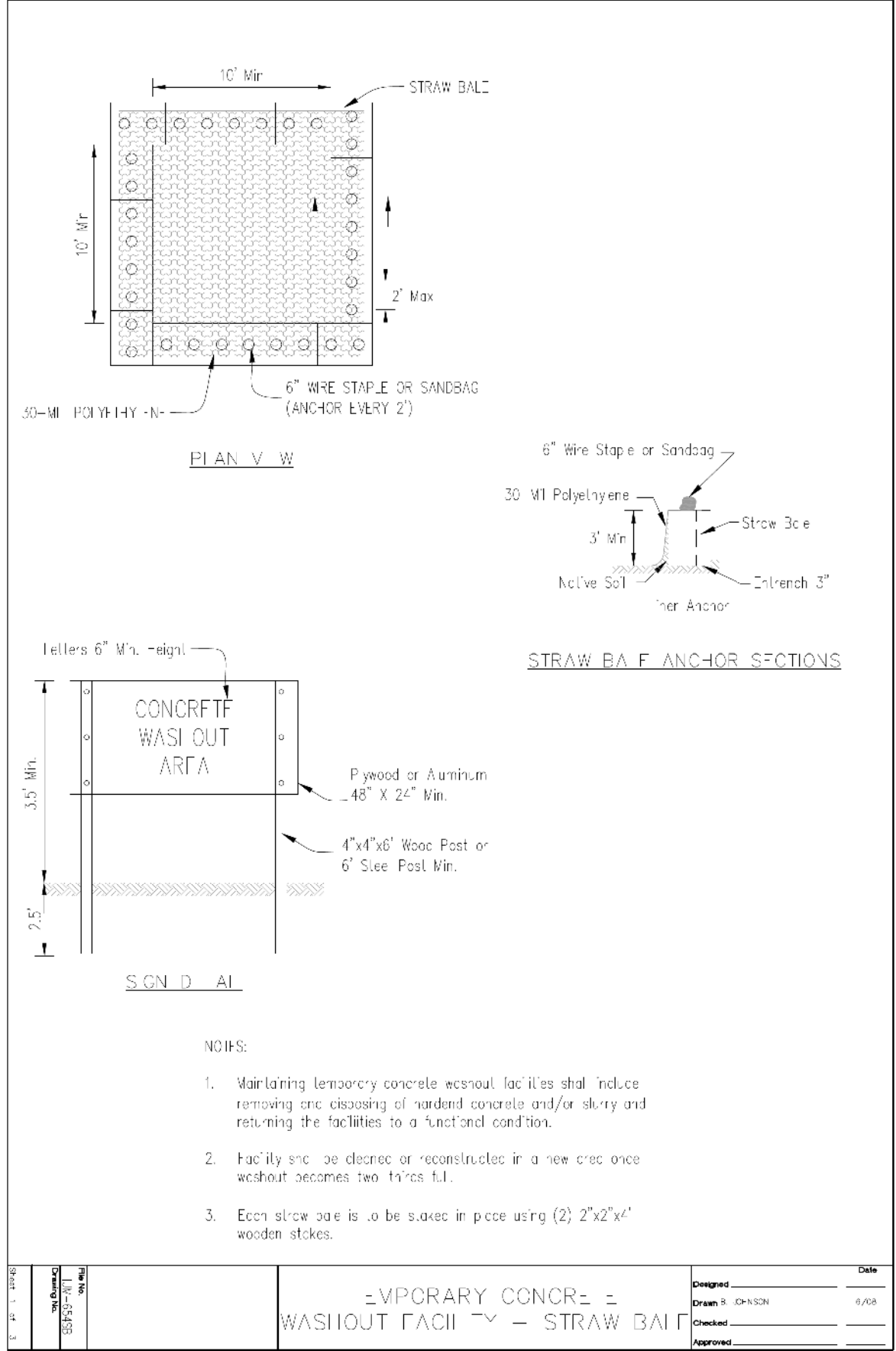
* IF A TYPE III BARRICADE WITH AN ATTACHED SIGN PANEL WHICH MEETS NCHRP 350 REQUIREMENTS IS NOT AVAILABLE, THE SIGNS SHALL BE MOUNTED, ABOVE THE BARRICADES, ON SEPARATE SIGNS SUPPORTS THAT MEET NCHRP 350 REQUIREMENTS.

USER NAME = faotomj DESIGNED - DRAWN - CHECKED - DATE -	DESIGNED - DRAWN - CHECKED - DATE -	REVISED - 10-18-02 REVISED - R. BORO 09-14-09 REVISED - REVISED -	STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION	DETOUR SIGNING FOR CLOSING STATE HIGHWAYS	F.A. RTE. SECTION COUNTY TOTAL SHEETS SHEET NO.
PLOT SCALE = 30,000' / in. PLOT DATE = 1/4/2019			SCALE: NONE SHEET 1 OF 1 SHEETS STA. TO STA.	TC-21 CONTRACT NO.	ILLINOIS T&A PROJECT

USER NAME = AlexSc DESIGNED - AMS DRAWN - CJH CHECKED - AJS DATE -	DESIGNED - DRAWN - CHECKED - DATE -	REVISED - REVISED - REVISED - REVISED -	CITY OF WHEATON DUPAGE COUNTY, ILLINOIS	CREEKSIDE DRIVE OVER SPRING BROOK NO. 1 DISTRICT STANDARDS	MS RTE. MARKED ROUTE COUNTY TOTAL SHEETS SHEET NO.
			SCALE: NTS SHEET 7 OF 8 SHEETS STA. N/A TO STA. N/A	4065 CREEKSIDE DRIVE DUPAGE 47 42	

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 PLOT DATE: 1/31/2023

AUT-CAT006



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 SHEET: 1 of 3

USER NAME = AlexSc	DESIGNED - AMS	REVISED -
PLOT SCALE = 2,000' / in.	DRAWN - CJH	REVISED -
PLOT DATE = 1/31/2023	CHECKED - AJS	REVISED -
	DATE -	REVISED -

**CITY OF WHEATON
 DUPAGE COUNTY, ILLINOIS**

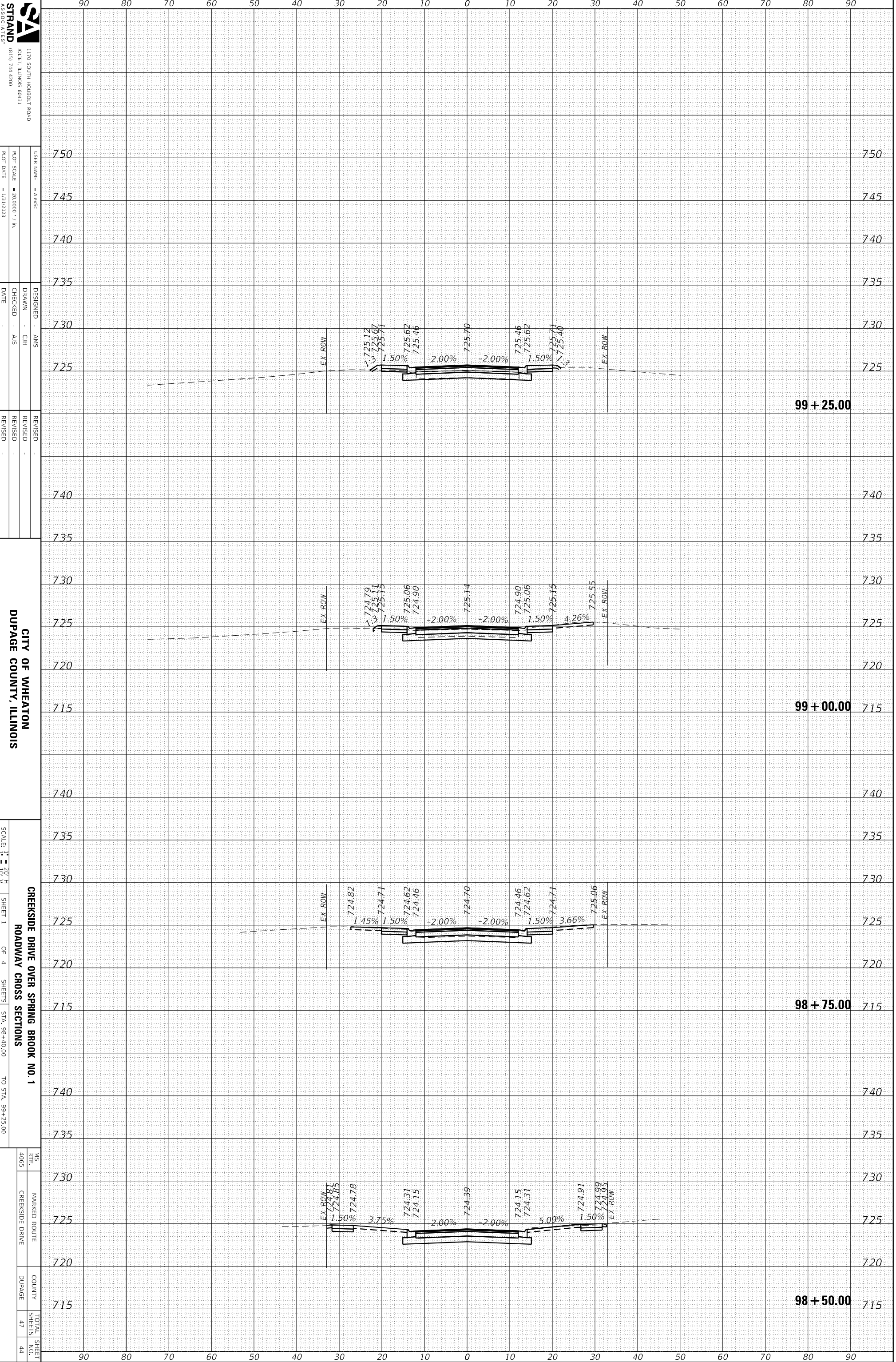
CREEKSIDE DRIVE OVER SPRING BROOK NO. 1			
ILLINOIS URBAN MANUAL STANDARDS			
SCALE: NTS	SHEET 8 OF 8 SHEETS	STA. N/A TO STA. N/A	

MS RTE.	MARKED ROUTE	COUNTY	TOTAL SHEETS	SHEET NO.
4065	CREEKSIDE DRIVE	DUPAGE	47	43

ORIGINAL SURVEY	SURVEYED _____	BY _____	DATE _____
NOTE BOOK	PLOTTED _____		
	TEMPLATE _____		
	AREAS _____		
	AREAS CHECKED _____		

FINAL SURVEY	SURVEYED _____	BY _____	DATE _____
NOTE BOOK	PLOTTED _____		
	TEMPLATE _____		
	AREAS _____		
	AREAS CHECKED _____		

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STRAND ASSOCIATES
 1170 SOUTH HOOBOLT ROAD
 JOULET, ILLINOIS 60431
 (815) 744-4200

USER NAME = AHSK
 DRAWN = CJH
 CHECKED = AIS
 DATE =

DESIGNED = AMS
 REVISIONS:
 REVISION NO. | DATE | DESCRIPTION

**CITY OF WHEATON
 DUPAGE COUNTY, ILLINOIS**

**CREKESIDE DRIVE OVER SPRING BROOK NO. 1
 ROADWAY CROSS SECTIONS**

SCALE: 1" = 30' H
 SHEET 1 OF 4 SHEETS STA. 98+40.00 TO STA. 99+25.00

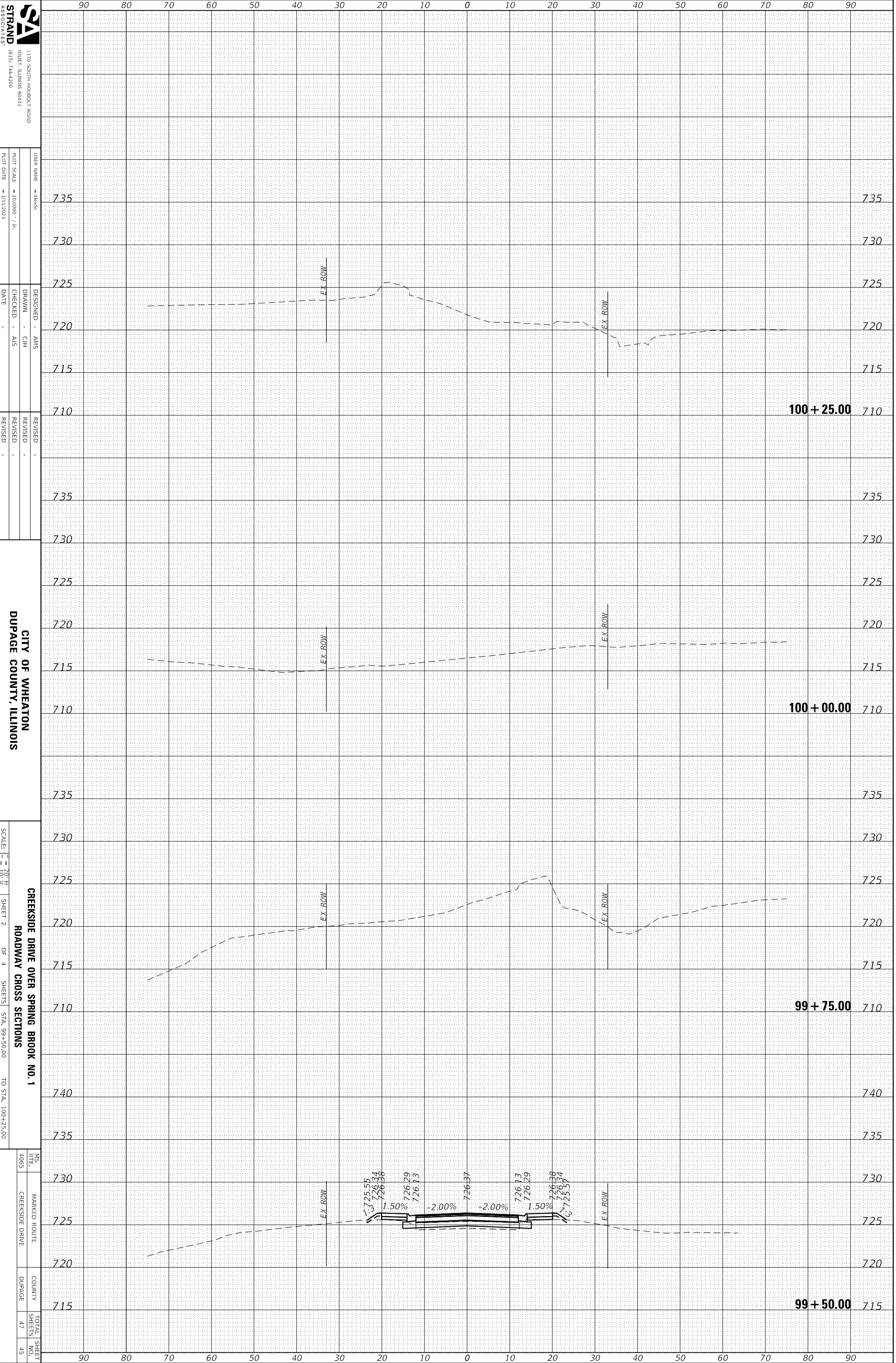
750	750
745	745
740	740
735	735
730	730
725	725
	99 + 25.00
740	740
735	735
730	730
725	725
720	720
715	99 + 00.00
740	740
735	735
730	730
725	725
720	720
715	98 + 75.00
740	740
735	735
730	730
725	725
720	720
715	98 + 50.00

NS. RTE. 4065
 MARKED ROUTE CREKESIDE DRIVE
 COUNTY DUPAGE
 TOTAL SHEET NO. 44
 SHEETS 47

ORIGINAL SURVEY	SURVEYED _____	BY _____	DATE _____
NOTE BOOK	PLOTTED _____		
	TEMPLATE _____		
	AREAS _____		
	AREAS CHECKED _____		

FINAL SURVEY	SURVEYED _____	BY _____	DATE _____
NOTE BOOK	PLOTTED _____		
	TEMPLATE _____		
	AREAS _____		
	AREAS CHECKED _____		

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STRAND ASSOCIATES
 1170 SOUTH HOOBOLT ROAD
 JOULET, ILLINOIS 60431
 (815) 744-4200

USER NAME = AhasC
 DESIGNED - AMS
 DRAWN - CJH
 CHECKED - AIS
 DATE -

REVISIONS
 REVISED -
 REVISED -
 REVISED -

CITY OF WHEATON
 DUPAGE COUNTY, ILLINOIS

100 + 25.00

100 + 00.00

99 + 75.00

99 + 50.00

SCALE: 1" = 30' H, 1" = 10' V
 SHEET 2 OF 4 SHEETS STA. 99+50.00 TO STA. 100+25.00

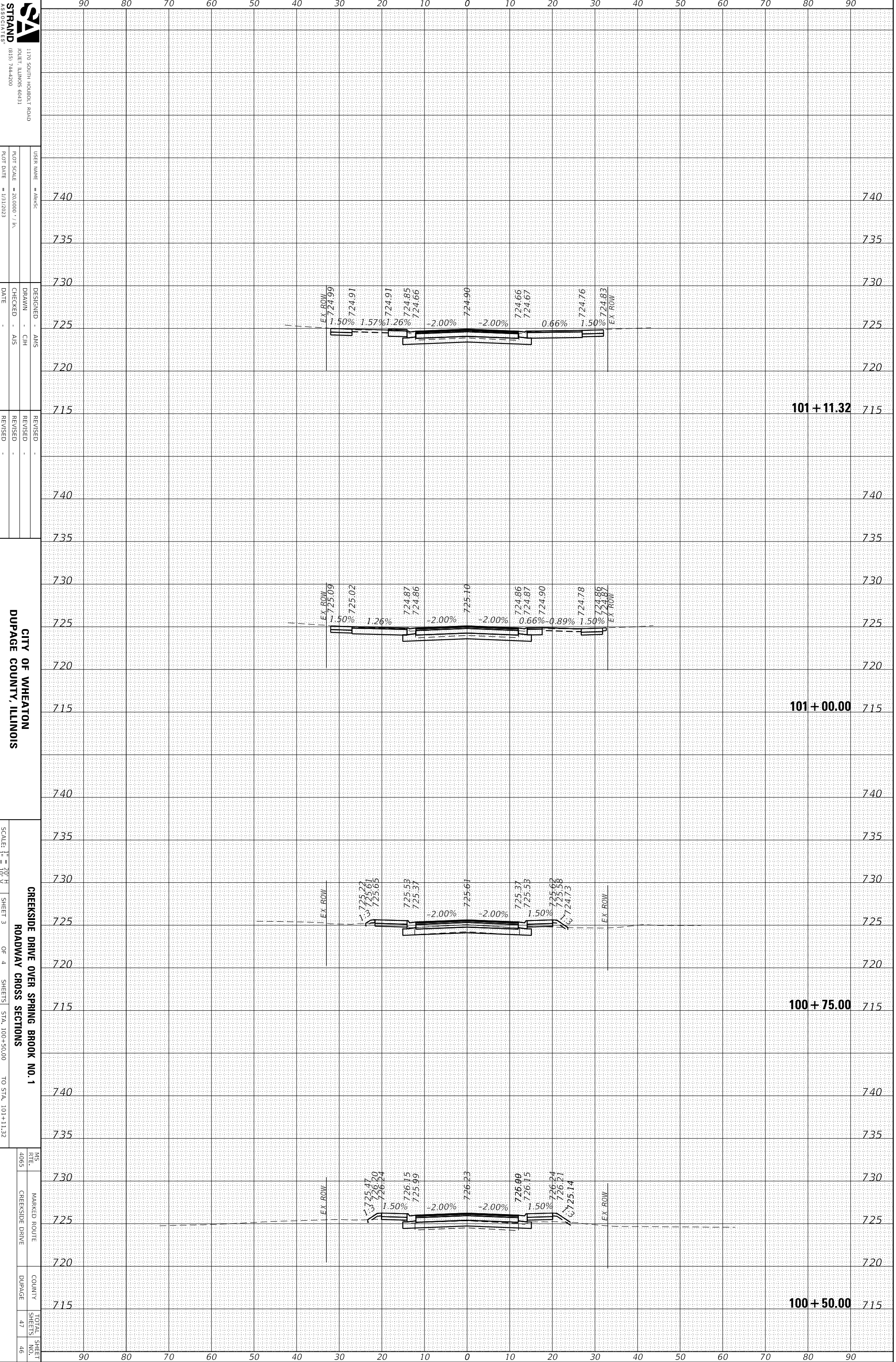
CREEKSIDE DRIVE OVER SPRING BROOK NO. 1
 ROADWAY CROSS SECTIONS

MS. RTE. 4065
 MARKED ROUTE CREEKSIDE DRIVE
 COUNTY DUPAGE
 TOTAL SHEET SHEETS NO. 47 OF 45

ORIGINAL SURVEY	SURVEYED	BY	DATE
NOTE BOOK	PLOTTED		
	TEMPLATE		
	AREAS		
	AREAS CHECKED		

FINAL SURVEY	SURVEYED	BY	DATE
NOTE BOOK	PLOTTED		
	TEMPLATE		
	AREAS		
	AREAS CHECKED		

MODEL: Default
 FILE NAME: S:\JUL1100-1199\1122\013\Drawings\CAD\Micro-SS4\CAD_Sheets\1122013-sht-XS.dgn



STRAND ASSOCIATES
 1170 SOUTH HOOBOLT ROAD
 JOULET, ILLINOIS 60431
 (815) 744-4200

USER NAME = AHS
 DESIGNED - AMS
 DRAWN - CJH
 CHECKED - AIS
 DATE -

REVISIONS
 REVISED -
 REVISED -
 REVISED -

**CITY OF WHEATON
 DUPAGE COUNTY, ILLINOIS**

SCALE: 1" = 30'
 SHEET 3 OF 4 SHEETS
 STA. 100+50.00 TO STA. 101+11.32

**CREKESIDE DRIVE OVER SPRING BROOK NO. 1
 ROADWAY CROSS SECTIONS**

NS. RTE. 4065
 MARKED ROUTE CREKESIDE DRIVE
 COUNTY DUPAGE
 TOTAL SHEET NO. 47
 SHEETS 46

ORIGINAL SURVEY	SURVEYED _____	BY _____	DATE _____
NOTE BOOK	PLOTTED _____		
	TEMPLATE _____		
	AREAS _____		
	AREAS CHECKED _____		

FINAL SURVEY	SURVEYED _____	BY _____	DATE _____
NOTE BOOK	PLOTTED _____		
	TEMPLATE _____		
	AREAS _____		
	AREAS CHECKED _____		

MODEL: Default
 FILE NAME: S:\JOL\1100-1199\1122\013\Drawings\CAD\Micros-SS4\CAD_Sheets\1122013-sht-XS.dgn

STRAND ASSOCIATES
 1170 SOUTH HOBBOLT ROAD
 JOULET, ILLINOIS 60431
 (815) 744-4200

USER NAME = AMSC
 PLOT SCALE = 20.0000' = 1" = 1/20'
 PLOT DATE = 1/31/2023

DESIGNED - AMS
 DRAWN - CJH
 CHECKED - AIS
 DATE -

REVISED -
 REVISED -
 REVISED -

**CITY OF WHEATON
 DUPAGE COUNTY, ILLINOIS**

SCALE: 1" = 20' H
 1" = 10' V

**CREKESIDE DRIVE OVER SPRING BROOK NO. 1
 ROADWAY CROSS SECTIONS**
 SHEET 4 OF 4 SHEETS STA. 101+25.00 TO STA. 101+50.00

MS. RTE. 4065	MARKED ROUTE	COUNTY	TOTAL SHEET NO.
	CREKESIDE DRIVE	DUPAGE	47
			47

